

Multi-Axial Hybrid Fire Testing based on Dynamic Relaxation

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Abstract

This technical note presents the experimental validation of a hybrid fire testing coordination algorithm recently developed by some of the authors. For the first time, the algorithm is applied to solve the static response of a multiple-degrees-of-freedom hybrid model.

Keywords: Hybrid fire testing, dynamic relaxation, partitioned time integration.

1. Introduction

Hybrid fire testing (HFT) replaces a prototype structure under test with a hybrid model that combines physical and numerical substructures (PS and NS, respectively). The PS is tested in the laboratory using servo-controlled actuators and furnaces. The NS is instantiated in a structural analysis software. A coordination algorithm ensures kinematic compatibility and force balance between PS and NS. There is a general consensus among researchers that coordination algorithms should rely on a constant PS tangent stiffness matrix estimate, possibly equal to its initial value, and facilitate the reuse of existing structural analysis software for simulating the NS. Consistent with this philosophy, a HFT coordination algorithm was recently proposed by Abbiati *et al.* [1], which relies on dynamic relaxation and partitioned time integration. The experimental validation of the algorithm on a single-degree-of-freedom (single-DoF) hybrid model was recently carried out at the Danish Institute of Fire Safety and Security Technology by Sauca *et al.* [2].

This technical note reports the results of the experimental validation of the same HFT algorithm on a multiple-DoFs hybrid model.

2. Coordination algorithm

The HFT algorithm proposed in [1] combines the dynamic relaxation method and partitioned time integration to solve the static coupled equation of motion of the hybrid model. The basic idea behind dynamic relaxation is to obtain the displacement solution

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of a static structural problem by computing the transient response of an equivalent dynamic system. For a hybrid model comprising a single PS and a single NS, the system of coupled equations of motion reads,

$$\begin{cases} \mathbf{M}^N \ddot{\mathbf{u}}^N + \mathbf{C}^N \dot{\mathbf{u}}^N + \mathbf{r}^N(\mathbf{u}^N) = \mathbf{f}^N + \mathbf{L}^{N^T} \boldsymbol{\Lambda}^N \\ \mathbf{L}^N \dot{\mathbf{u}}^N + \bar{\mathbf{L}}^N \dot{\mathbf{u}}^g = \mathbf{0} \\ \mathbf{M}^P \ddot{\mathbf{u}}^P + \mathbf{C}^P \dot{\mathbf{u}}^P + \mathbf{r}^P(\mathbf{u}^P) = \mathbf{f}^P + \mathbf{L}^{P^T} \boldsymbol{\Lambda}^P \\ \mathbf{L}^P \dot{\mathbf{u}}^P + \bar{\mathbf{L}}^P \dot{\mathbf{u}}^g = \mathbf{0} \\ \bar{\mathbf{L}}^{N^T} \boldsymbol{\Lambda}^N + \bar{\mathbf{L}}^{P^T} \boldsymbol{\Lambda}^P = \mathbf{0} \end{cases} \quad (1)$$

where $\mathbf{r}$ are internal restoring force vectors while $\mathbf{f}(t)$ represent time-varying external loading; $\mathbf{M}$ and $\mathbf{C}$ are fictitious mass and damping diagonal matrices, which are defined as,

$$M_{ii} = \frac{(1.1\Delta t)^2}{4} \sum_j |K_{ij}|, \quad C_{ii} = 2\omega_0 M_{ii} \quad (2)$$

where K_{ij} is a generic element of the initial tangent stiffness matrix $\mathbf{K} = \partial \mathbf{r} / \partial \mathbf{u}$; Δt is the time step size of the equivalent transient analysis; ω_0 is the lower eigenvalue of the coupled equations of equilibrium, as defined in (1), which is estimated with a linearized monolithic version of the hybrid model. In order to cast compatibility and equilibrium equations among substructures, $\mathbf{L}$ and $\bar{\mathbf{L}}$, which are Boolean signed matrices, localize interface DoFs on every single substructure and vector $\mathbf{u}_g$, respectively. The latter gathers all DoFs that are shared between at least two substructures, namely the interface DoFs.

3. Experimental validation

The experimental validation of the coordination algorithm relies on the 3-DoFs prototype structure reported in Figure 1, which consists of a simply supported beam with elastic restraints. Specifically, u_1 , u_2 and u_3 indicate the axial and the two rotational DoFs of the hybrid model while r_1 , r_2 and r_3 , the corresponding restoring force and bending moments. The PS coincides with the beam element and consists of an aluminium plate of $b \times h = 200 \times 2$ mm rectangular cross-section and $L = 460$ mm length. The main material parameters of aluminum are Young modulus $E = 69.50$ GPa, density $\rho = 2700$ kg/m³, thermal expansion $\alpha = 23 \times 10^{-6}$ C⁻¹, thermal conductivity $\lambda = 201$ W/mC, specific heat $c_p = 910$ J/kgC. The NS coincides with the elastic restraints, that is, the lateral spring of stiffness $K_1 = 2500.00$ kN/mm and both rotational spring of stiffness $K_2 = 876.25$ and $K_3 = 1052.50$ kNmm/rad. Thermal loading is imposed to the PS by means of a 2.4 kW infrared lamp module. In detail, the temperature measured at the center of the plate is linearly ramped from the room temperature $\theta_0 \approx 20$ C to $\theta_{\max} = 135$ C in about 1400 s. The following hold phase lasts for 600 s. Noteworthy, the stiffness of the axial spring K_1 is about twenty times larger than the axial stiffness of the plate. As a result, the latter tends to buckle if subjected to thermal expansion. Figure 2 provides an overview of the experimental setup, for which a comprehensive description is reported in [3].

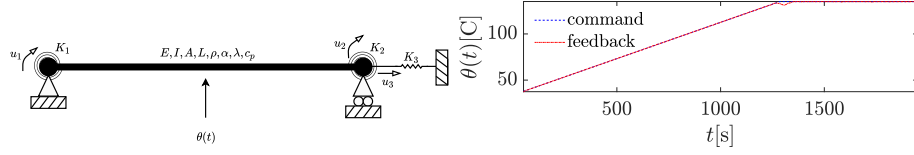


Figure 1: Validation case study: a) prototype structure; b) temperature history.



Figure 2: Experimental setup: a) 3-DoFs test rig; b) plate specimen; c) infrared lamp.

4. Results and discussion

The effectiveness of the algorithm is demonstrated by examining response quantities related to a single HFT. In this regard, Figure 3 compares PS and NS displacement response quantities while Figure 4 compares PS and NS force response quantities. For the sake of comparison, in the latter case, PS response quantities are multiplied by -1 so that matching with NS response quantities can be graphically verified. As one can note, some discontinuity characterizes both u_3 and r_3 , which are associated with the right rotational DoF of the hybrid model. The reason is that the bearings of the right clevis of the 3-DoFs test rig are not perfectly aligned and the joint suffers from stick and slip behavior as reported in [3]. However, as can be observed, both compatibility and balance between PS and NS are attained with a high degree of accuracy. Finally, for the sake of comparison, residual forces, which are computed as the sum of fictitious inertia and damping forces of both PS and NS, are reported in Figure 5. As can be observed, residual forces are small compared to restoring forces of Figure 4.

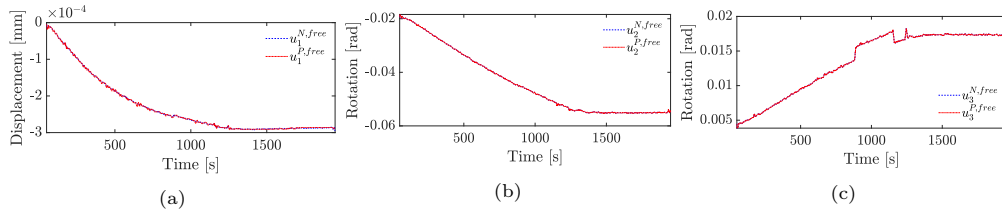


Figure 3: Displacement response a) displacement DoF u_1 ; b) rotation DoF u_2 ; c) rotation DoF u_3 .

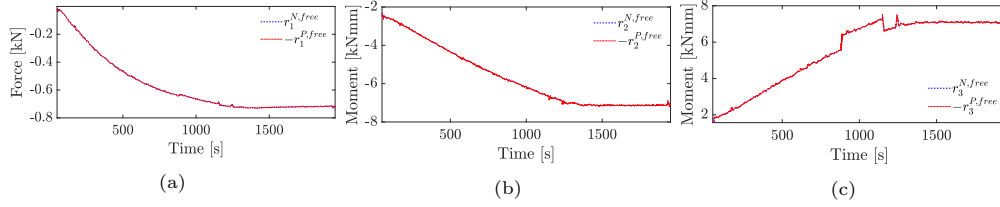


Figure 4: Restoring forces: a) displacement DoF u_1 ; b) rotation DoF u_2 ; c) rotation DoF u_3 .

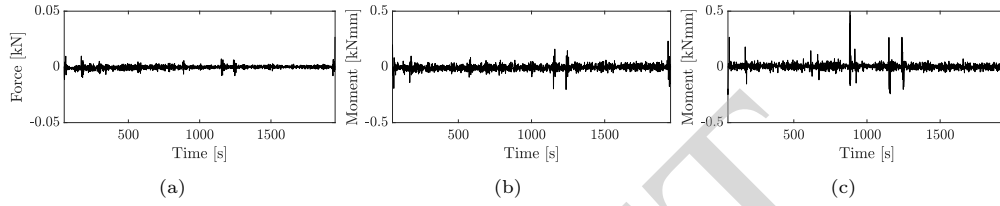


Figure 5: Residual forces: a) displacement DoF u_1 ; b) rotation DoF u_2 ; c) rotation DoF u_3 .

5. Conclusions and outlooks

Experimental results demonstrate that, with the proposed coordination algorithm, compatibility and balance between physical and numerical substructures are attained with a high level of accuracy. Also, residual forces are small compared to restoring forces. Hybrid fire testing with nonlinear numerical substructures will be the target of our future work.

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