

1 **Effect of saturation procedures on direct simple shear testing of silt tailings**

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26 **Abstract**

27 The direct simple shear (DSS) test carried out under constant volume (CV) conditions forms one
28 of the primary laboratory techniques to characterise soils and tailings. The use of CV
29 conditions to simulate undrained shearing is supported by historical evidence on the testing of
30 a saturated clay and sands, with this evidence being incorporated into current guidelines and
31 state of practice procedures. However, some recent comparisons of the results of undrained
32 hollow cylinder simple shear (HCSS) and CV DSS tests on predominately silt gold tailings
33 adopting state of practice test procedures (i.e., inundation of the sample after loose moist
34 tamping) showed much less post-peak strength loss in the gold tailings than the undrained
35 HCSS tests. The current study investigated this discrepancy further by carrying out DSS tests
36 under high back pressures, undrained simple shear tests with flexible membrane and constant
37 cell pressure (FM-CCP) and DSS tests after flushing with carbon dioxide and with use of a small
38 back pressure. In all cases, the undrained tests or DSS tests with greater effort put towards
39 saturation exhibited greater post-peak strength loss more consistent with the HCSS and the
40 critical state line. The importance of these results on the estimation of tailings brittleness in
41 engineering practice was outlined.

42

43

44 **Introduction**

45 The direct simple shear (DSS) forms one of the key laboratory tests for the monotonic and cyclic
46 characterisation of soils. An important aspect of common DSS testing is the use of the
47 constant volume (CV) approach to reproduce undrained shearing conditions. There is strong
48 evidence that for saturated clays CV tests produce results indistinguishable from undrained
49 shearing (Dyvik et al. 1987). Further, for sands, it has been found that the CV technique allows
50 tests on dry sands to give the same results as saturated sands (Finn and Vaid 1977), with further
51 confirmation provided by more recent comparisons of moist tamped sands with and without
52 inundation with water (Kaviani-Hamedani et al. 2025, Sadrekarimi and Abharian 2025). Further,
53 procedures for the testing of dry or moist (unsaturated) sands are now being extended more
54 generally for non-plastic soils (Chen et al. 2022) and adopted in determination of the critical
55 state line (CSL) by means of CV DSS tests (Chen and Olson 2021). However, it is noted that for
56 cyclic testing of clayey sands this similitude between dry and saturated samples has been
57 questioned (Monkul et al. 2015). Further, Fanni et al. (2022) showed that simple shear tests
58 carried out in a hollow cylinder torsional shear system (HCSS tests) showed far more brittle
59 behaviour than CV DSS tests of the same gold tailings at similar states, raising questions about
60 the efficacy of conventional saturation procedures in silts.

61 Mine tailings, which comprise a wide range of material types and gradation from sands to clays,
62 often consisting predominantly of low plasticity silts, are increasingly being characterised using
63 CV DSS (e.g. Jefferies et al. 2019). Such tests are often prepared initially unsaturated using
64 loose moist tamping (LMT) and saturated through flushing, followed by keeping samples in
65 contact with water at atmospheric pressure. Current standards for CV DSS testing, and
66 specifically their requirements around sample saturation (or not) are as follows:

- 67 • Cyclic DSS testing (ASTM 2019): testing of dry or saturated samples are permitted, with
68 saturated samples to be “inundat[ed] with water”.

69 • Monotonic testing of fine grained soils (ASTM 2017), which does not include mention of
70 testing dry samples, and also requires saturated samples to be “inundat[ed] with
71 water”.

72 As noted, the current standards either permit testing dry samples or mandate inundation.
73 However, as emphasised by the results of Monkul et al. (2015) and Fanni et al. (2022) there
74 remains uncertainty as to whether testing tailings silts dry, or even saturated through flushing
75 and then keeping the sample in contact with water at atmospheric pressure (i.e. “inundation”),
76 will ensure reliable results. The purpose of the current study is to expand on the work of Fanni
77 et al. (2022) to further investigate this issue and provide preliminary recommendations on
78 means to improve the monotonic response of LMT-prepared tailings silts in CV DSS tests. The
79 work is examined within the context of critical state soil mechanics (Jefferies and Been 2015),
80 particular with respect to the tendency (or not) to tend to the critical state line (CSL) at high
81 strain.

82 **Materials and methods**

83 ***Materials***

84 The study was carried out on a low plasticity gold tailings previously used by Fanni et al. (2022),
85 having been characterised in a number of parallel studies (Reid et al. 2022; Ayala et al. 2022;
86 Fanni et al. 2024) and a low plasticity platinum tailings batch previously used for cyclic and
87 limited monotonic testing (Reid et al. 2024; Reid and Fanni 2024) with the critical state line
88 (CSL) obtained by Reid (2022). The index properties and CSL of the two materials are
89 summarised in Table 1.

90 ***Table 1: Index properties of soils tested in this study***

Property	Gold tailings	Platinum tailings
% < 75 µm	59	72
% < 38 µm	43	60

Liquid limit (%)	18	18
Plastic limit (%)	16	12
Plasticity Index	2	6
Specific Gravity (-)	2.78	3.04

91

92 ***Conventional CV DSS approach***

93 The first set of tests were carried out on gold tailings using “conventional” CV DSS testing to
 94 confirm and expand the results of Fanni et al (2022). The tests carried out are summarised in
 95 Table 1. Samples of 100mm diameter were prepared in an SGI DSS manufactured by GDS
 96 Instruments. Active CV control was applied by means of a linear variable displacement
 97 transducer (LVDT) mounted directly to the top cap, recording displacement on a smooth
 98 stainless steel “track” mounted to the base of the bottom platen.

99 Samples were prepared using LMT, then placed within the DSS and a bedding load ranging from
 100 25 - 500 kPa applied, with the variation in saturation bedding load used to target different loose
 101 densities as per Reid et al. (2023). The samples were flushed from bottom to top with deionised
 102 water until bubbles ceased exiting the top of the specimen and saturation collapse as tracked
 103 through vertical displacement of the sample ceased. Both ends of the sample were then
 104 connected to the deionised water reservoir placed at the same height as the sample - i.e., an
 105 approach to saturation that is consistent with typical industry practice for the testing of moist
 106 tamped tailings silts (Jefferies et al. 2019) and aligns with the requirement for “inundation” of
 107 samples as per ASTM (2017). Samples were then consolidated to the target vertical effective
 108 stress, prior to CV shearing at 5%/hour. Active height control was carried out throughout the
 109 shearing process with a maximum displacement recorded variation of 2µm in the tests carried
 110 out in this study, well within the typical 0.05% requirements (ASTM 2017, 2019).

111 Void ratio for CV DSS tests were calculated based on (i) dry mass of solids, and (ii) sample
 112 volume, calculated based on the diameter of the specimen and height, where height was

113 obtained by means of a relationship developed between the displacement values of an internal
114 “encoder” LVDT and vertical load, which incorporates system compressibility in the calculation
115 of sample height. It is noted that this internal encoder LVDT used for sample height calculation
116 is distinct from that used for the maintenance of constant height. Initial tamped specimen
117 heights were approximately 25 mm, which after saturation collapse and consolidation were
118 <20mm, representing a maximum H/D ratio of 5, consistent with typical recommendations to
119 minimise stress non-uniformity in DSS tests (e.g. Amipour et al. 2022).

120 **Table 2: Summary of tests carried out on gold tailings in current study and Fanni et al. (2022)**

Test no.	Test type	Test source	Consolidated state				Shearing response			
			Vertical effective stress, σ'_{vc} (kPa)	Drained static shear stress, τ_{vh-c} (kPa)	Consol. void ratio, e_c	ψ_0	Peak τ_{vh} (kPa)	σ'_v at end of shear (kPa)	τ_{vh} at end of shear	I_B
G-DSS1	Inundate only	Fanni et al. (2022)	300	0	0.70	0.09	46	63	37	0.20
G-DSS2		Fanni et al. (2022)	300	0	0.70	0.09	47	61	37	0.20
G-DSS3		Fanni et al. (2022)	200	0	0.72	0.10	30	37	23	0.23
G-DSS4		Fanni et al. (2022)	496	0	0.69	0.11	76	117	59	0.22
G-DSS5		This study	200	0	0.72	0.10	28	45	22	0.23
G-FMCCP-1	FM-CCP	This study	202	0	0.71	0.09	29	3	5	0.81
G-DSS-500BP-1	500 kPa BP	This study	199	0	0.69	0.07	29	9	10	0.66

121

122 ***Flexible membrane with constant cell pressure***

123 A second set of tests on the gold tailings were carried out in a simple shear system that allows
124 cell pressure confinement (e.g. Mele 2023), with the device used in the current study
125 manufactured by GDS. This form of testing was outlined by Boulanger et al. (1993) and is
126 commonly referred to as “Berkeley-type” simple shear. In the current work, adopting the
127 terminology of Airey and Acharya (2025) we refer to this test method as flexible membrane with
128 constant cell pressure (FM-CCP). Although alternative test procedures, such as maintaining a
129 constant vertical total stress are sometimes adopted, herein we use FM-CCP (constant cell
130 pressure) as this appears to provide the results most consistent to CV DSS as noted by Airey
131 and Acharya. The FM-CCP adopted in the current study did not feature internal measurement of
132 radial displacement as adopted in some versions of this testing (Kang and Kang 2015, Kang et
133 al. 2015, Kang et al. 2016).

134 Samples were prepared using LMT within a membrane-lined split mould. Initial flushing was
135 carried out under 50 kPa vertical bedding load with the split mould still in place. After
136 saturation collapse ceased, a suction of 20 kPa was applied to the sample using a
137 pressure/volume controller and the split mould was removed. This process allowed the sample
138 diameter to be accurately measured after saturation collapse, removing the most significant
139 uncertainty in calculating sample volume. The samples were then back pressure (BP) saturated
140 to 500 kPa, achieving a minimum B value of 0.95.

141 Consolidation was carried out to a vertical effective stress of 200 kPa while maintaining a K_0 of
142 0.5, then the drainage valves closed, and the sample sheared undrained at 5%/hour. During
143 shearing, the cell pressure was held constant. End of test soil freezing was not viewed as a
144 practical approach for the FM-CCP tests carried out owing to the relatively large size of the filter
145 stones compared to the sample size, which could potentially result in significant errors from

146 “contamination” of pore fluid not originally part of the sample. This differs from triaxials with
147 lubricated ends, where the sample is proportionally much larger than the small filter stones.

148 ***CV DSS with 500 kPa BP***

149 Another test on the gold tailings was carried out using the CV DSS approach with lateral
150 confinement provided by Teflon rings, but with the entire sample within the system able to
151 apply cell pressure confinement, enabling a BP of 500 kPa, resulting in a B value of 0.95.

152 ***CV DSS with small BP***

153 Given the promising results seen in the FM-CCP and BP CV DSS tests on the gold tailings,
154 another form of CV DSS was carried out that could be performed in a conventional
155 apparatus. This was to produce a method of improved saturation and more realistic monotonic
156 behaviour in a system far more common in industry than the FM-CCP. This set of tests was
157 carried out on the platinum tailings, with the test on this material summarised in Table 3.

158

159 **Table 3: Summary of Platinum tailings CV DSS tests carried out in this study and Reid and Fanni (2024)**

Test no.	Test type	Test source	Consolidated state				Shearing response			
			Vertical effective stress, σ'_{vc} (kPa)	Drained static shear stress, τ_{vh-c} (kPa)	Consol. void ratio, e_c	ψ_0	Peak τ_{vh} (kPa)	σ'_v at end of shear (kPa)	τ_{vh} at end of shear	I_B
P-IU-1	Inundate only	This study	200	0	0.72	0.10	29	32	16	0.44
P-IU-2		This study	200	0	0.75	0.13	27	32	16	0.39
P-IU-3		This study	200	0	0.79	0.17	25	24	11	0.56
P-IU-4		Reid and Fanni (2024)	250	25	0.73	0.12	45	52	26	0.43
P-IU-5		Reid and Fanni (2024)	250	25	0.73	0.11	46	50	26	0.43
P-IU-6		This study	250	25	0.70	0.09	45	47	25	0.44
P-IU-7		This study	500	0	0.66	0.09	76	97	54	0.29
P-SAT-1	CO2 flush and 10kPa BP	This study	250	25	0.67	0.05	45	25	13	0.72
P-SAT-2		This study	250	25	0.71	0.10	40	16	7	0.83
P-SAT-3		This study	500	0	0.66	0.08	71	29	16	0.77

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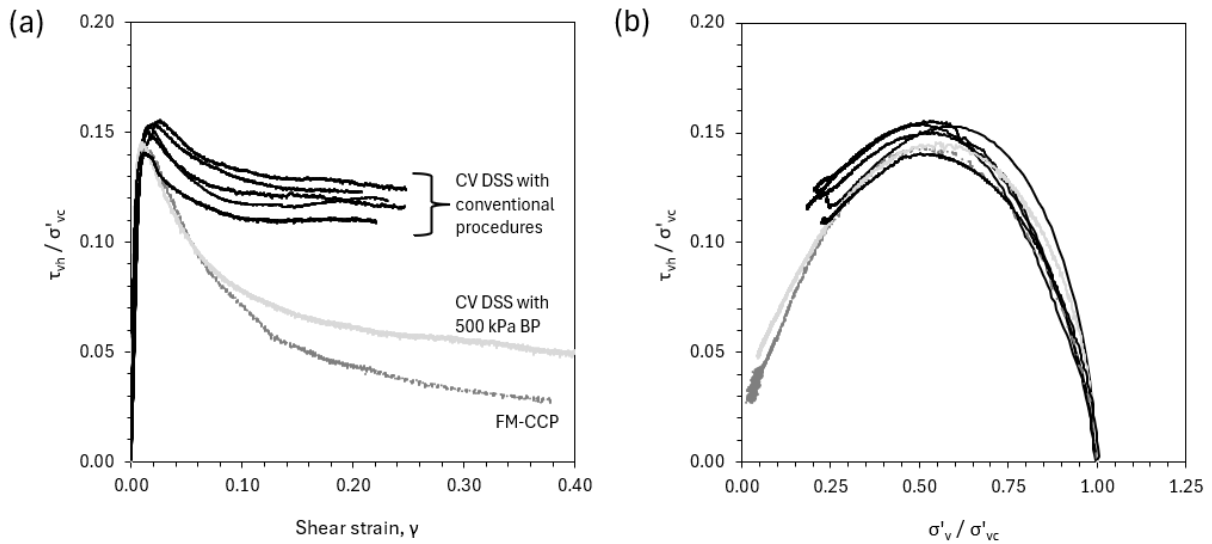
161

162 The modified procedure involved LMT preparation and application of a bedding load of 50 kPa,
163 followed by flushing with CO₂ and then deaired water. After bubbles had ceased exiting the
164 sample and saturation collapse was complete (same criteria as standard CV DSS tests
165 previously outlined), the reservoir of deaired water was placed 1m above the sample and
166 connected to both ends of the specimen to provide a 10 kPa BP. It was found that with use of
167 four o-rings to secure the membrane to the top platen, this magnitude of BP could be applied
168 without leakage. Although it is acknowledged that 10 kPa BP is unlikely to produce a
169 meaningful improvement to saturation quality, it was viewed as of merit in any case as
170 potentially improving the uniformity of the distribution of moisture within the sample.

171 **Results**

172 ***Gold tailings***

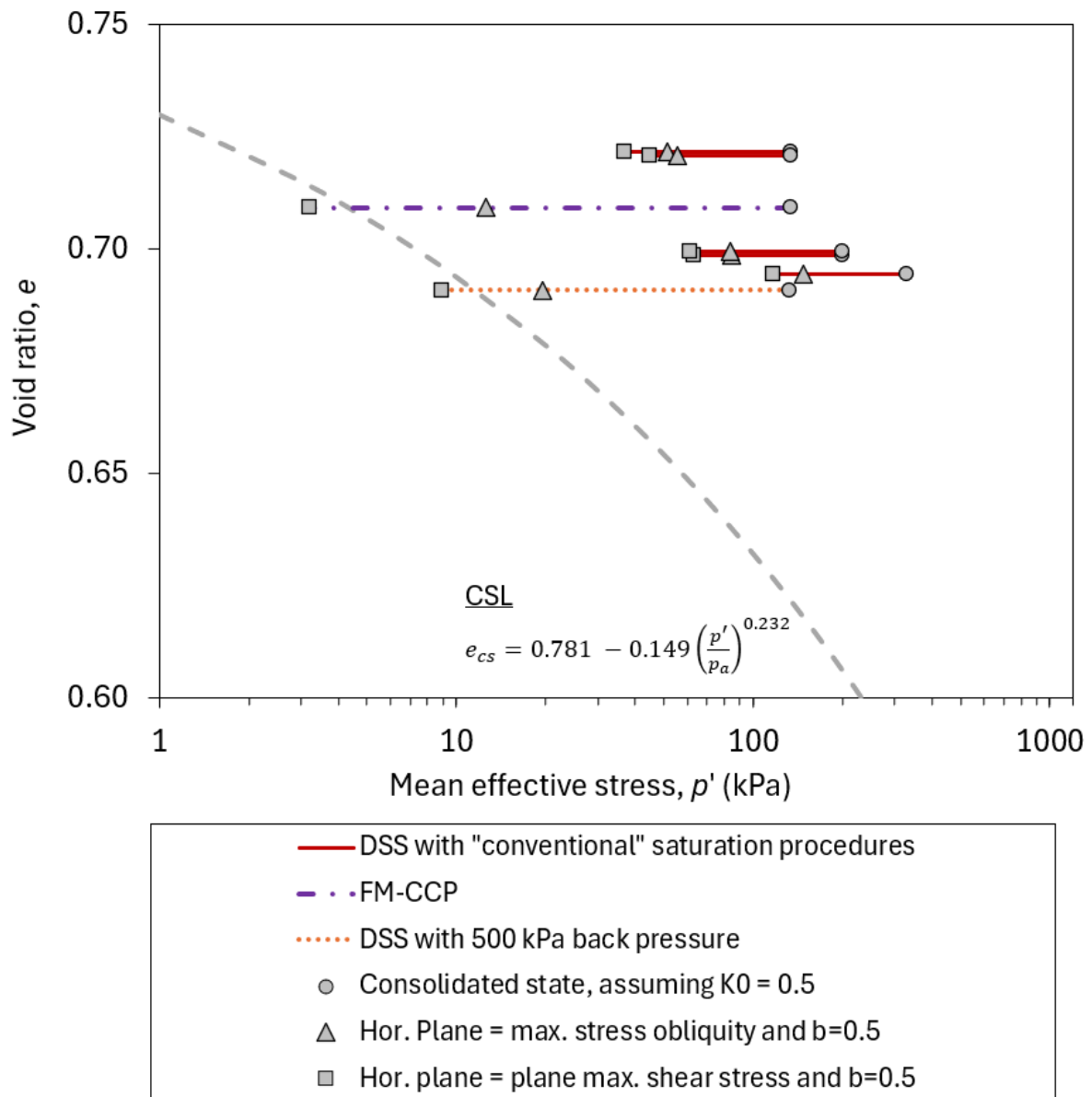
173 A summary of the gold tailings shearing behaviour as shear stress-strain and vertical effective
174 stress - shear stress plots are provided in Figure 1. The response of the FM-CCP tests and CV
175 DSS test with BP are seen to be qualitatively different to those using standard saturation
176 procedures despite similar consolidated void ratios (e_c) - a far greater post-peak reduction in
177 strength is evident, consistent with the differences seen between CV DSS and HCSS in the
178 previous comparison presented by Fanni et al. (2022). All the conventional CV DSS tests
179 appear to achieve a steady state condition at high strain that would typically be assumed to
180 represent a critical state condition in such testing (e.g. Chen and Olson 2021). The CV DSS with
181 BP and FM-CCP are still slightly softening at high strain yet may be approaching a critical state
182 condition.



183

184 **Figure 1: Summary of gold tailings CV DSS and FM-CCP tests: (a) horizontal shear stress vs**
 185 **shear strain, (b) horizontal shear stress vs. vertical effective stress**

186 The results of the gold tailings tests are summarised as a state diagram in Figure 2, with the CSL
 187 obtained by Fanni et al. (2024) included for reference. Calculation of initial mean effective
 188 stress (p') assumed a K_0 of 0.5. In recognition of the uncertainty in calculating p' in the DSS at
 189 high strain, based on Chen and Olson (2021) maximum and minimum principal effective
 190 stresses σ'_1 and σ'_3 were calculated assuming: (i) that the horizontal plane was equal to
 191 maximum obliquity or (ii) that the horizontal plane was the plane of maximum shear
 192 stress. Intermediate principal effective stress σ'_2 was calculated assuming that the
 193 intermediate principal stress ratio at high strain was 0.5. Based on these assumptions, at high
 194 strain FM-CCP and CV DSS with BP appear to tend much closer towards CSL for the gold
 195 tailings obtained from triaxial tests, in contrast to the conventional CV DSS tests. It is
 196 emphasised that for this gold tailings there is no evidence for a Lode angle dependency on CSL
 197 elevation in the compression plane (Fanni et al. 2024), and thus no reason why shearing under
 198 simple shear loading should not tend towards the triaxial-determined CSL.



199

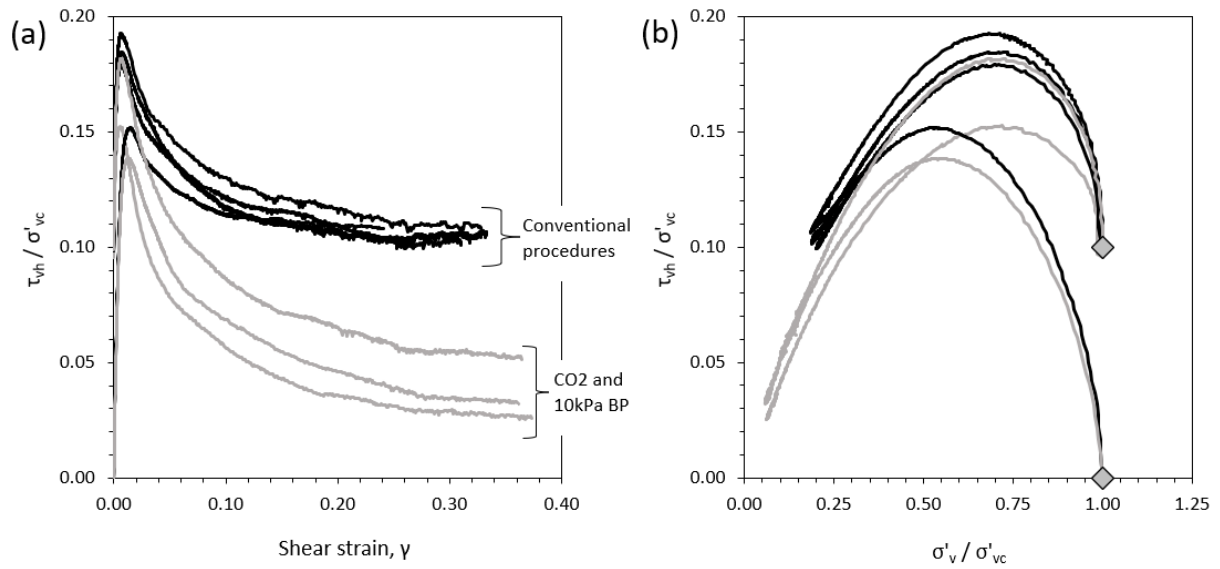
200 **Figure 2: State diagram of gold tailings CV DSS and FM-CCP tests, with CSL Fanni et al.**

201 **(2024)**

202 **Platinum tailings**

203 The shearing response of the platinum tailings is summarised in Figure 3. Like the gold tailings,
 204 the samples with BP exhibited a qualitatively more brittle response than conventional tests,
 205 with the end points of all tests appearing to represent a critical state condition.

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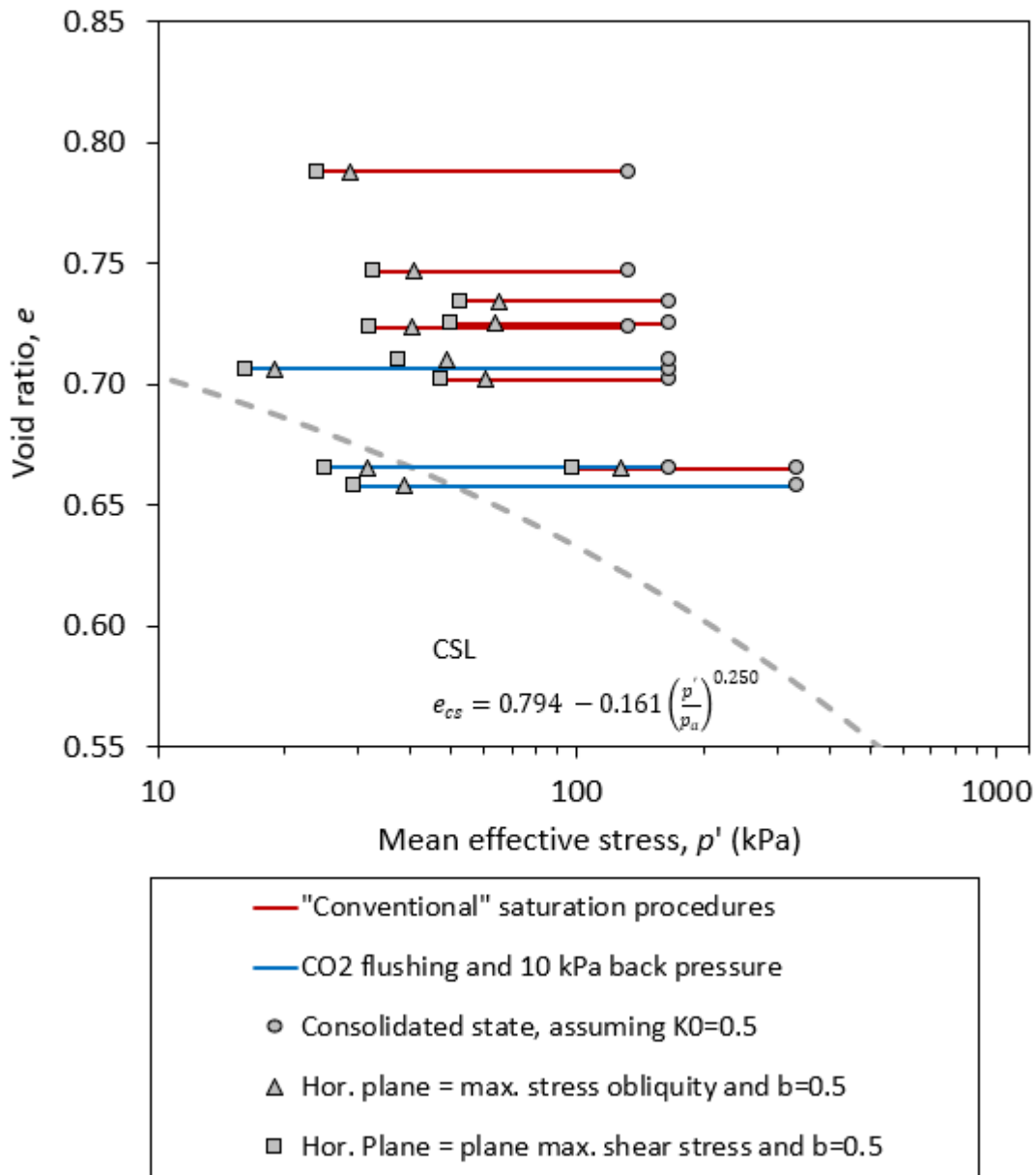


207

208 **Figure 3: Summary of platinum tailings CV DSS: (a) horizontal shear stress vs shear strain,**
 209 **(b) horizontal shear stress vs. vertical effective stress**

210 The platinum tailings tests are summarised as a state diagram in Figure 4 using the same
 211 assumptions as the previous gold tailings plot. The results are qualitatively similar - tests with
 212 CO₂ flushing and 10 kPa BP align far better with the triaxial-inferred CSL than those using
 213 conventional saturation procedures. It is noted that the tests with CO₂ flushing and 10 kPa BP
 214 generally achieve slightly denser states, likely as a result of the improved saturation and
 215 resulting slight increase in saturation collapse.

216



217

218 **Figure 4: State diagram of platinum tailings CV DSS tests, with CSL from Reid (2022)**

219 As to the likely cause of the response seen, insufficient saturation of conventional tests may
 220 mean some matric suction remains, and thus the samples are tending to a CSL at a higher
 221 elevation (e.g. Wheeler and Sivakumar 1995). It is noted that for predominantly silt tailings
 222 prepared using LMT there is a greater potential for matric suction to exist and thus for suction-
 223 hardening of the CSL than clean sands. Therefore, historic testing of dry or moist sands, and
 224 the approaches developed largely from the conclusions drawn therein, may not be useful
 225 guides as to the importance of saturation in the CV DSS testing of tailings silts. Indeed, in

226 recent comparisons of moist and saturated sand DSS tests, the negligible matric suction in
227 such cases was the primary reason suggested for the indistinguishable results in that case
228 (Sadrekarimi and Abharian 2025).

229 ***Brittleness index synthesis***

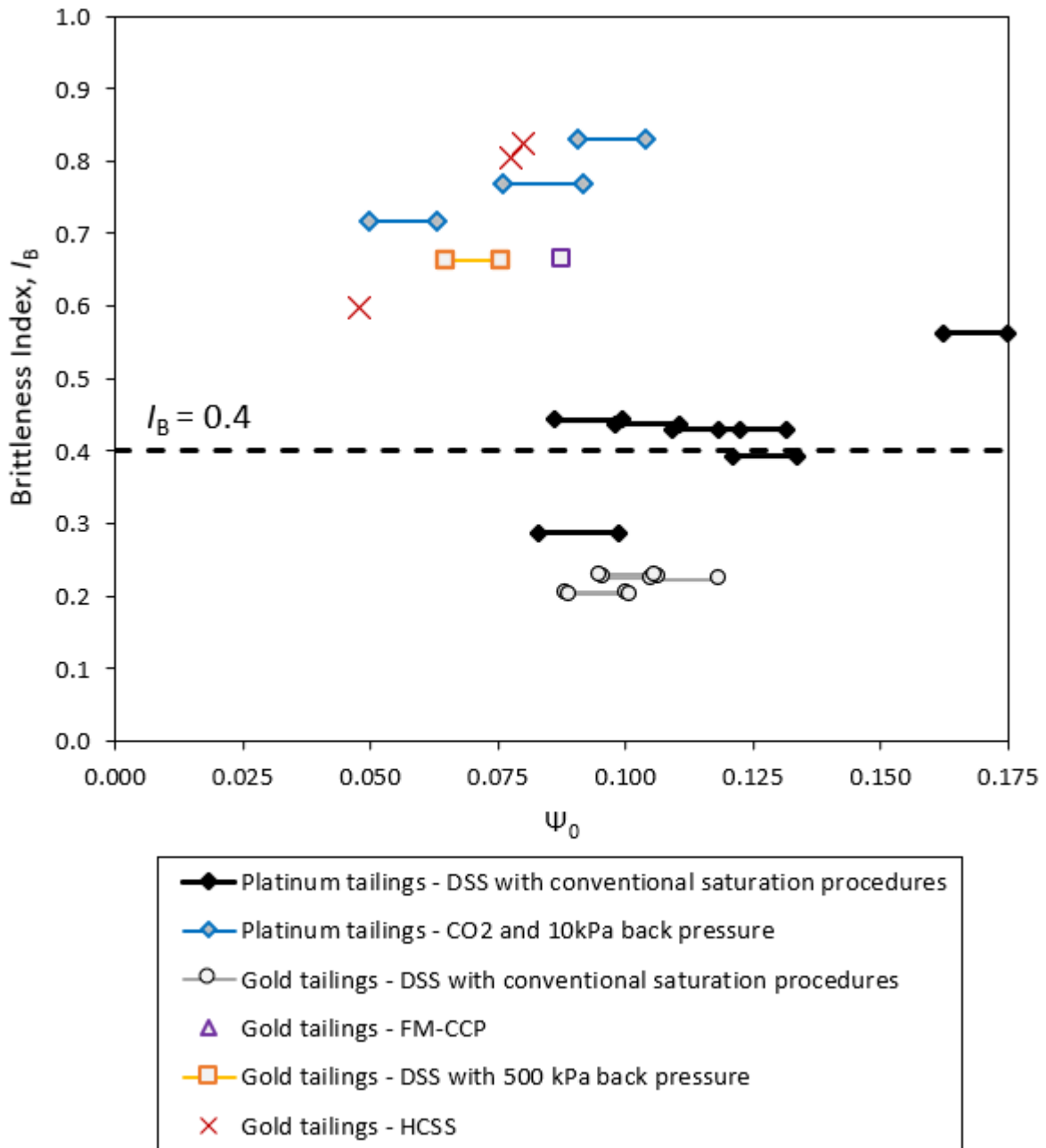
230 To examine the practical implications of use of conventional-saturation CV DSS on engineering
231 practice, Figure 5 presents a synthesis of all the results from the study as Brittleness Index
232 I_B (Bishop 1967) against consolidated state parameter. For CV DSS tests where K_0 is not
233 measured, the results are presented showing a range of plausible K_0 values from 0.4 to 0.7.

234 For reference, I_B is calculated as:

$$235 \quad I_B = \frac{\tau_p - \tau_r}{\tau_p}$$

236 Where τ_p and τ_r are peak and residual shear strength, respectively. An I_B value of 0.4 is also
237 highlighted based on the work of Robertson (2017) showing that historic examples of flow
238 liquefaction are generally of greater I_B . It can be seen from this synthesis that tests utilising
239 improved saturation techniques exhibit far greater brittleness at a particular value of Ψ_0 , with
240 these agreeing well with the results of previous HCSS testing. Further, the lesser brittleness of
241 loose samples in conventional CV DSS could lead to potentially erroneous conclusions
242 regarding the risk associated with a particular deposit of tailings (e.g. ICOLD 2023) – for
243 example, the likely erroneous values of I_B seen for conventional CV DSS would result in different
244 recommended minimum Factor of Safety values adopting the current ICOLD guidelines.

245



246

247 **Figure 5: Synthesis of Brittleness Index for all tests considered in this study. For DSS tests**
 248 **where p'_c is uncertain, Ψ_0 calculated assuming a range from K_0 0.4 to 0.7.**

249 **Conclusions**

250 The current study involved a more detailed investigation of previously-documented issues with
 251 the large strain monotonic CV DSS shearing response of LMT-prepared silt tailings. This was

252 carried out by further conventional and back-pressure saturated CV DSS tests and FM-CCP
253 tests on a gold and platinum tailings. The results of the study can be summarised as follows:

- 254 • CV DSS tests saturated using methods that represent current standard/conventional
255 approaches (i.e. “inundation”) show less post-peak loss of strength and contractive
256 tendency than implied by their initial state.
- 257 • FM-CCP and CV DSS tests with varying levels of BP saturation show a behaviour more
258 consistent with the sample initial states, tending much closer to the CSL.
- 259 • The results strongly suggest that a more accurate monotonic large-strain
260 characterisation of silt tailings is achieved in the CV DSS if greater efforts are made to
261 improve saturation compared to that specified in current testing standards and
262 common industry methods.
- 263 • The reduced I_b seen from conventional CV DSS tests could lead to erroneous
264 conclusions regarding the risk of flow liquefaction of loose tailings deposits.

265 **Data availability statement**

266 Data generated or analysed during this study are available in the OSF repository at

267 DOI 10.17605/OSF.IO/EU8PG, direct link:

268 https://osf.io/eu8pg/?view_only=aae417a84eba40a6879ace035120980a

269 **Material availability statement**

270 Material used for testing in this study are available from the corresponding author upon

271 reasonable request.

272 **References**

273 Airey, D and Acharya, B. 2025. Influence of boundary conditions on undrained simple shear

274 tests. *Geotechnical Testing Journal* 48(4).

275 Amipour, A. Khashila, M., Bayoumi, A., Karray, M. and Chekired, M. 2022. Specimens size effect
276 D/H on cyclic behaviour and liquefaction potential of clean sand. *Acta Geotechnica* 17.

277 ASTM International. 2017. Standard Test Method for Consolidated Undrained Direct Simple
278 Shear Testing of Fine Grain Soils. West Conshohocken, PA: ASTM International.

279 ASTM International. 2019. Standard Test Method for Consolidated Undrained Cyclic Direct
280 Simple Shear Test under Constant Volume with Load Control or Displacement Control.
281 West Conshohocken, PA: ASTM International.

282 Ayala, J., Fourie, A., and Reid, D. 2022. Improved Cone Penetration Test Predictions of the State
283 Parameter of Loose Mine Tailings. *Canadian Geotechnical Journal* 59 (11): 1969–80.

284 Bishop, A. W. 1967. Progressive Failure-with Special Reference to the Mechanism Causing It.
285 *Proc. Geotech. Conf.*, Oslo 2:142–50.

286 Boulanger, R., Chan, C., Seed, H., Seed, R., and Sousa, J.. 1993. A Low-Compliance Bi-
287 Directional Cyclic Simple Shear Apparatus. *Geotechnical Testing Journal* 16(1).

288 Chen, J, Olson, S.M., Banerjee, S., Dewoolkar, M.M., and Dubief, Y.. 2022. Water Content of
289 Moist-Tamped Nonplastic Specimens for Constant-Volume Direct Simple Shear Testing.
290 *Geotechnical Testing Journal* 45 (2): 20210125.

291 Chen, J., and Olson, S.M. 2021. SHANSEP-Based Interpretation of Overconsolidation Effect on
292 Monotonic Shearing Resistance of Contractive Nonplastic Soils. *Journal of Geotechnical
293 and Geoenvironmental Engineering* 147 (12): 04021155.

294 Dyvik, R., Berre, T., Lacasse, S., and Raadim, B. 1987. Comparison of Truly Undrained and
295 Constant Volume Direct Simple Shear Tests. *Géotechnique* 37 (1): 3–10.

296 Fanni, R., Reid, D., and Fourie, A.. 2022. On Reliability of Inferring Liquefied Shear Strengths
297 from Simple Shear Testing. *Soils and Foundations* 62 (3): 101151.

298 Fanni, R., Reid, D., Fourie, A. 2024. Drained and Undrained Behaviour of a Sandy Silt Gold
299 Tailings under General Multiaxial Conditions. *Géotechnique*. Ahead of print.
300 <https://doi.org/10.1680/jgeot.23.00186>.

301 Finn, W. D. L., and Y. P. Vaid. 1977. Liquefaction Potential from Drained Constant Volume cyclic
302 Simple Shear Tests. In *Proceedings of the Sixth World Conference on Earthquake*
303 *Engineering*, 2157–62.

304 ICOLD. 2023. Tailings Dam Safety. Bulletin 194. Paris, France: ICOLD.

305 Jefferies, M. and Been, K. 2015. Soil liquefaction - a critical state approach. Second Edition.
306 CRC Press.

307 Jefferies, M., Morgenstern, N.R., Van Zyl, D.V., and Wates, J. 2019. Report on NTSF
308 Embankment Failure, Cadia Valley Operations, for Ashurst Australia. Australia, Ashurst.

309 Kang, X. and Kang, G.C. 2015. Modified monotonic simple shear tests on silica sand. *Marine*
310 *Geosciences and Geotechnology* 33(2).

311 Kang, X., Cheng, Y., Louis, G. 2015. Radial Strain Behaviors and Stress State Interpretation of
312 Soil Under Direct Simple Shear. *Journal of testing and evaluation* 43(6).

313 Kang, X., Ge, L., Chang, K.T. and Kwok, A.O. 2015. Strain-Controlled Cyclic Simple Shear Tests
314 on Sand with Radial Strain Measurements. *Journal of Materials in Civil Engineering* 28(4).

315 Kaviani-Hamedani, F., Fakharian, K., Shabani, F., Khoshghalb, A., Shirkavand, D. and Baghbani,
316 A. 2025. On the effects of multi-directional initial anisotropy on the monotonic constant
317 volume response of a silica sand. *Canadian Geotechnical Journal* 62.

318 Mele, L. 2023. “Experimental study with complete stress state interpretation of undrained
319 monotonic and cyclic simple shear tests with flexible boundaries.” *Acta Geotech.* 19 (1):
320 147–161.

321 Monkul, M.M., Gültekin, C., Gülver, M., Akın, O., and Eseller-Bayat, E. 2015. Estimation of
322 Liquefaction Potential from Dry and Saturated Sandy Soils under Drained Constant
323 Volume Cyclic Simple Shear Loading. *Soil Dynamics and Earthquake Engineering* 75
324 (August):27–36.

325 Morgenstern, N. R., Vick, S.G., and Van Zyl, D.V. 2015. Report on Mount Polley Tailings Storage
326 Facility Breach. Report of Independent Expert Engineering Investigation and Review Panel.
327 Prepared on behalf of the Government of British Columbia and the Williams Lake and
328 Soda Creek Indian Bands.

329 Reid, D. 2022. Slope Stress Prediction Benchmarking Exercise: Triaxial Calibration Data. Slope
330 Stress Prediction Benchmarking Exercise: Triaxial Calibration Data.
331 <https://doi.org/10.17605/OSF.IO/RTJ3P>.

332 Reid, D., Fanni, R., and Fourie, A.B. 2024. Drained Static Bias Effects on Very Loose Silt Tailings.
333 *Japanese Geotechnical Society Special Publication 10 (27)*: 995–1000.

334 Reid, D., and Fanni, R. 2024. Discussion of ‘Flow Failure Assessments for Dams and
335 Embankments’ by T.D Stark, J. Lin and H. Jung. *Canadian Geotechnical Journal*. Under
336 review.

337 Reid, D., Fanni, R., and Fourie, A. 2022. Effect of Tamping Conditions on the Shear Strength of
338 Tailings. *International Journal of Geomechanics* 22 (3): 04021288.

339 Reid, D., Urbina, F., Tiwari, B., Fanni, R., Smith, K., and Fourie, A. 2023. Effect of Saturation
340 Confining Pressure on Accessible Densities and Shear Behaviour of a Sandy Silt Tailings.
341 *Géotechnique Letters* 13 (2): 1–5.

342 Robertson, P. K. 2017. Evaluation of Flow Liquefaction: Influence of High Stresses. *PBD III*,
343 *Earthquake Geotechnical Engineering*. Vancouver, B.C.

- 344 Sadrekarimi, A. and Abharian, S. Effect of specimen size, saturation, and loading caps in
345 monotonic direct simple shear testing of a sand. *Geotechnical Testing Journal* 48(4).
- 346 Wheeler, S. J., and Sivakumar, V.. 1995. An Elasto-Plastic Critical State Framework for
347 Unsaturated Soil. *Géotechnique* 45 (1): 35–53.