

# Making the Black Box Transparent: State of the Art in Explainable Machine Learning for Structural Design and Assessment

Mohsen Zaker Esteghamati<sup>1</sup>, Jingcheng Wang<sup>2</sup>, Xiaowei Wang<sup>3</sup>, Stephanie G. Paal<sup>4</sup>, Jacob Murphy<sup>5</sup>, Ali Namin<sup>6</sup>, Abdullahi Salman<sup>7</sup>, Adam Ado Sabari<sup>8</sup>, Muhammad Ahsan Ibrar<sup>9</sup>, Ram Mazumder<sup>10</sup>, Yue Li<sup>11</sup>, Abdollah Shafieezadeh<sup>12</sup>

<sup>1</sup> Assistant Professor, Department of Civil and Environmental Engineering, Utah State University, Logan, UT, 84322, Email: mohsen.zaker@usu.edu (**Corresponding Author**, Address: 4110 Old Main Hill, Logan, UT, 84322)

<sup>2</sup> Associate Professor, College of Civil Engineering, Fuzhou University, Fuzhou 350108, China, Email: [jingchengwang@fzu.edu.cn](mailto:jingchengwang@fzu.edu.cn)

<sup>3</sup> Associate Professor, Department of Bridge Engineering, Tongji University, Shanghai 200092, China, Email: [xiaoweiwang@tongji.edu.cn](mailto:xiaoweiwang@tongji.edu.cn)

<sup>4</sup> Williams Brothers Construction Company Associate Professor, Zachry Department of Civil & Environmental Engineering, Texas A&M University, College Station, TX, 77840, Email: [spaal@civil.tamu.edu](mailto:spaal@civil.tamu.edu)

<sup>5</sup> Graduate Student, Zachry Department of Civil & Environmental Engineering, Texas A&M University, College Station, TX, 77840, Email: [jacob70920@tamu.edu](mailto:jacob70920@tamu.edu)

<sup>6</sup> Graduate Student Department of Civil and Environmental Engineering, Utah State University, Logan, UT, 84322, Email: [ali.namin@usu.edu](mailto:ali.namin@usu.edu)

<sup>7</sup> Associate Professor, Department of Civil and Environmental Engineering, University of Alabama in Huntsville, Huntsville, AL 35899. Email: [ams0098@uah.edu](mailto:ams0098@uah.edu)

<sup>8</sup> Graduate Student, Department of Civil and Environmental Engineering, University of Alabama in Huntsville, Huntsville, AL 35899. Email: [aas0041@uah.edu](mailto:aas0041@uah.edu)

<sup>9</sup> Graduate Student, Department of Civil and Environmental Engineering, University of Alabama in Huntsville, Huntsville, AL 35899. Email: [mi0025@uah.edu](mailto:mi0025@uah.edu)

<sup>10</sup> Assistant Professor, Department of Civil Engineering & Construction, Georgia Southern University, Statesboro, GA 30458. Email: [rmazumder@georgiasouthern.edu](mailto:rmazumder@georgiasouthern.edu)

<sup>11</sup> Leonard Case Jr. Professor in Engineering, Department of Civil and Environmental Engineering, Case Western Reserve University, Cleveland, OH 44106. Email: [yxl1566@case.edu](mailto:yxl1566@case.edu)

<sup>12</sup> Lichtenstein Professor, Department of Civil, Environmental and Geodetic Engineering, The Ohio State University, Columbus, OH 43210. Email: [shafieezadeh.1@osu.edu](mailto:shafieezadeh.1@osu.edu)

## Abstract

Machine learning (ML)-based solutions have gained traction in various structural engineering applications, from structural design to assessment and monitoring. Nevertheless, the black-box nature of advanced ML models, and the resultant limited interpretation and transparency, are among the primary barriers to their broader adoption and implementation in the field. eXplainable ML (XML) is an interdisciplinary field that improves the understanding of the ML model performance. Despite the potential of XML to increase the accessibility of ML, the scattered available literature and the lack of a domain-specific holistic review of XML have created a significant knowledge gap for its application in structural engineering. Therefore, this paper presents a targeted review of the XML definition, nomenclature and taxonomy, frequently used algorithms, and domain-specific literature. Additionally, three case studies are presented to illustrate different classes of XML algorithms and their implementation on diverse structural engineering problems at the component-, structure-, and inventory levels, providing insights into how these techniques can provide engineering-oriented interpretations that enhance understanding of the studied problems.

**Keywords:** *Explainable Machine learning; Artificial intelligence; Structural Engineering; Design and Assessment*

## 1. Introduction

The application of machine learning (ML) in structural engineering has accelerated in recent years, enabling advances in design and assessment of structures (Kareem 2020; Salehi and Burgueño 2018; [3–6]). More recently, researchers have begun to explore the use of eXplainable Machine Learning (XML) to tackle the opacity of complex ML models. XML provides a means to understand and interpret the output of ML systems, fostering a new form of human-ML collaboration [7,8]. The growing emphasis on causal inference [9], scientific discovery [10], and algorithmic fairness [11] further motivates the use of XML. Additionally, the emergence of legal requirements for ML-based systems, such as the “right to explanation”, suggests that XML may soon become a necessary component of ML-based solutions in engineering applications [12].

Among various XML concepts, *model transparency* is a key principle that supports users’ understanding and trust [7,13]. In this context, transparency refers to the extent to which a model’s inner workings can be explained and communicated [8], and is typically characterized along three different dimensions: simulatability (i.e., simple enough to be simulated by humans), decomposability (i.e., the ability to disintegrate a model into parts that can be explained), and algorithmic transparency [14]. While providing source code, dataset, and training procedure contributes to transparency, XML methods go further by providing direct user-oriented explanations, especially for those lacking technical expertise [15]. These explanations can then be delivered through various formats, including text, numerical output, visual representations, tabular data, speech, expressive motions and lights, or mixed forms [16].

XML offers transformative potential for meaningful breakthroughs in structural design and assessment by improving ML transparency, thereby overcoming key barriers to its broader adoption. However, despite the availability of various surveys on ML applications in the field [5,17–19], there is currently no literature that maps XML concepts, nomenclature, and algorithms to the domain-specific requirements of structural engineering. Therefore, this study, first, reviews XML concepts, classification taxonomies, and common algorithms. Next, through a targeted literature review and illustrative examples, the accrued knowledge is adapted and applied to topics in structural engineering, with the goal of increasing access to ML in the field through XML.

The paper is organized as follows: Section 2 presents a review of XML literature from related fields, providing a comprehensive overview of existing definitions and taxonomies, common algorithms, and their classification. Section 3 introduces the XML literature on structural engineering design and assessment at three levels: component, structure, and inventory. Section 4 provides three illustrative examples that cover different XML techniques applied at different scales. Lastly, Sections 5 and 6 conclude the paper by outlining future directions for research and the integration of XML in structural engineering, as well as a summary of the discussed material. The compiled information and illustrative examples aim to provide a curated resource for advanced practitioners and academics interested in XML, enabling them to identify and apply best practices in implementing these methods for their intended applications in the design and assessment of the built environment.

## 2. A Review of XML Definition, Taxonomies, and Techniques

### 2.1. Definition and nomenclature

XML does not have a single universally accepted definition due to its evolving nature and the diversity of methods in this field. For example, Grunning [33] defines XML as models that “*enable human users to understand, appropriately trust, and effectively manage the emerging generation of AI partners*”. In contrast, Arrieta et al. state “*given an audience, an explainable AI is one that produces details or reasons to make its functioning clear or easy to understand*” [20]. Even the definition of the term “explainability” varies across the literature. For example, Hu et al. [34] define an *explanation* as a group of statements that clarifies or provides a reason for an action or belief, and categorize explanations by their purpose, type, and presentation. Doshi-Velez and Kim [22] defined explainability as “*The currency in which we exchange beliefs*”. As shown in Table 1, numerous concepts are associated with XML, including transparency, understandability, comprehensibility, explicability, interpretability, and explainability [20–22,24,29–32]. In particular, *explainability* and *interpretability* are two terms that are sometimes used interchangeably, although some authors distinguish between them [10,22,24,35]. Rudin [36] defines *interpretability* as designing a model to be inherently transparent, whereas *explainability* provides post-hoc explanations for a black-box model. Accordingly, interpretability relates to the model’s internal structure, whereas explainability is associated with how users understand its outputs [37]. Lipton [38] frames this distinction as interpretability addresses “*How does the model work?*” whereas explainability asks “*What else can the model tell me?*”.

Developing fully interpretable ML models is challenging, as interpretability requires the ability to synthesize the model into understandable components and the transparency of the underlying algorithms [39]. Given the trade-off between model interpretability and flexibility, black-box models are often preferred for capturing complex patterns. Therefore, this paper primarily focuses on post-hoc explainability applicable across a wide range of ML models.

**Table 1.** Common nomenclature related to XML

Term	References	Definition and notes
Explainability	[20,21]	Explanation as an interface between AI and human; explanations should be comprehensible and accurate proxy of how model works; often is considered post-hoc
Interpretability	[22,23]	Degree which a human can understand the model and its purpose; often is considered an intrinsic property of the model
Transparency	[23,24]	The model by itself is understandable (does not require explanations)
Explicability	[21]	The properties of AI model are inspectable
Correctability /Corrigibility	[25–27]	AI model can be adapted by human agent in a manner that ensure its correctness; associated with AI robustness
Comprehensibility	[21,26,28]	The ability of the model to represent its learning in a human understandable fashion, focus on user-side cognitive load
fidelity	[29]	Accuracy of the explanations to. represent the original model behavior locally
Faithfulness	[30]	The degree to which explanations represent the original model internal reasoning; separates real from fake explanations
Decomposability	[24]	Ability to understand and inspect each component of the model
Controllability	[31]	The ability of user to steer, interact and refine the AI output
trustworthiness	[32]	the degree to which explanations improve user trust, a complex concept that combine understandability, fidelity and robustness

## 2.2. Explainability techniques classification

From a broad perspective, XML can be categorized as either ante-hoc or post-hoc methods [37]. Ante-hoc (i.e., “white-box”) models achieve transparency by design, such as linear regression or decision tree-based models. In contrast, post-hoc methods (“black-box” models) create approximations of the model’s prediction process to explain how model reaches its outcome [16]. Minh et al. [8] further refined this classification into pre-modeling explainability, interpretable models, and post-modeling explainability.

The post-hoc XML techniques are often categorized into two main groups: model-agnostic and model-specific methods. Model-agnostic approaches focus solely on the input-output behavior of the model [34], and can be applied to a variety of different ML algorithms. Model-agnostic techniques include explanation by simplification, feature relevance explanation, local explanation, architecture modification, explanation by example, and visual explanation [16]. On the other hand, model-specific approaches leverage the internal structure of models, such as gradients and activation [40]. Some authors suggest that post-hoc explainability can also be categorized as local or global approaches. Local explanations seek to explain individual predictions and are generally relevant to ML algorithms where the decision affects a person’s “right to explanation” ([34,41]). These methods offer advantages due to their higher clarity and lower computing cost [42]. In contrast, global models focus on the overall behavior of the ML model, and might be more computationally demanding [43].

## 2.3. Existing XML taxonomies and relevance to structural engineering

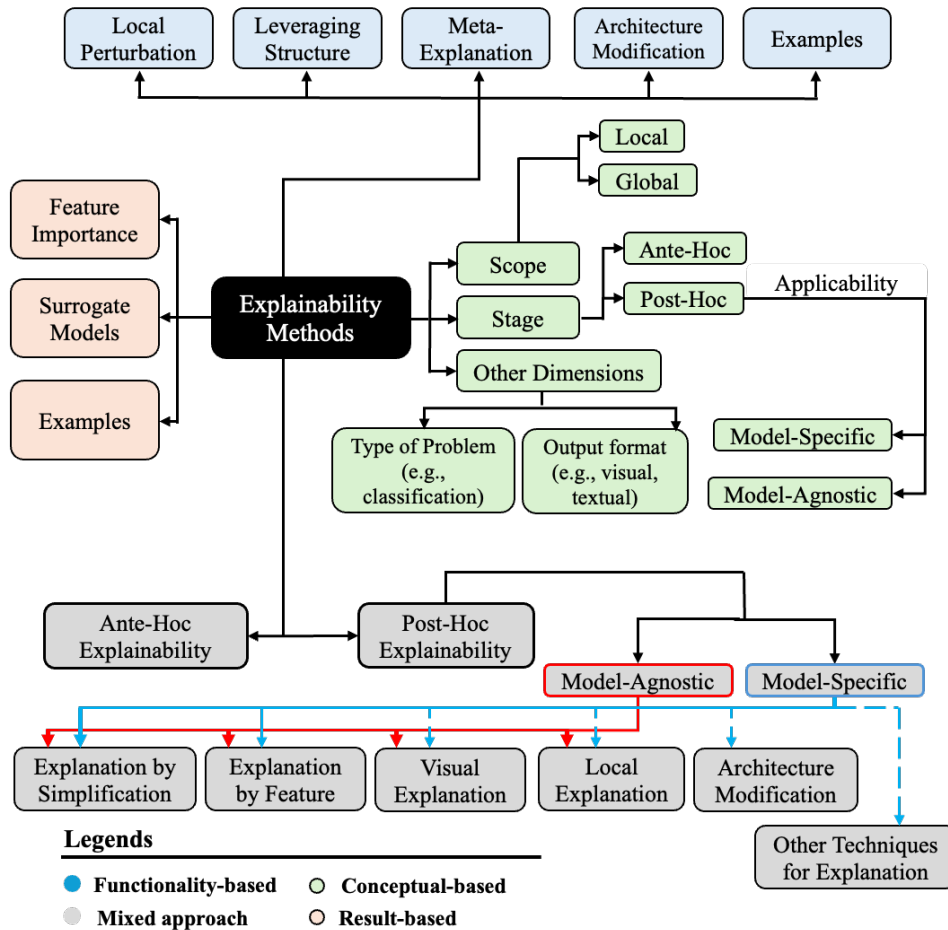
Different taxonomies are proposed to classify XML techniques based on scope, usage, or methodology. Figure 1 shows several common taxonomies, namely functionality-based, conceptual, result-based, and mixed taxonomies [44], whereas Table 2 compares the advantages and disadvantages of each presented taxonomy.

The functionality-based taxonomy [20,45] focuses on how XML techniques extract information from a model and includes local perturbation, leveraging structure, meta-explanation, architecture modification, and examples. Local perturbations evaluate how small input changes affect response to identify the most influential input, often using gradients. Leveraging structure-based methods use model architecture, particularly in neural networks, to generate explanations by encoding the model’s internal connections and layers into the explanation process. For example, the layer-wise relevance propagation technique [46] tracks information flow through the network layers from the output to the input. Lastly, meta-explanations identify simple user-centric patterns by examining and synthesizing multiple model explanations [47]. For example, spectral relevance analysis [48] clusters individual heatmaps to analyze the model predictive capabilities.

Results-based taxonomies [49] classify XML based on the nature of their output, typically derived from functioning-based categories. The three main categories are feature importance, surrogate models, and example-based methods. Feature importance methods quantify the influence of each input on model predictions, often by calculating the average marginal contribution of the feature across all possible combinations [50]. Surrogate models are simplified models that approximate the behavior of more complex models, such as Local Interpretable Model-agnostic Explanation (LIME) [29,51]. Examples-based methods use specific instances to demonstrate how the model works, including counterfactuals (how different inputs would change the outcome), adversarial examples (how small changes can cause

**Table 2.** Comparison of different existing taxonomies for XML

Taxonomy	Strength	Weakness
Functionality-based	Insights on internal mechanisms of XML techniques; easy to extend	Focus on inner-workings may prevent broader view on model output when used in isolation
Result-based	Organized by output, user-friendly (e.g. practitioners can select XML technique based on the form of explanation needed)	Lacks details on how models work, potential to miss on methodological nuances on relationship between different techniques
Conceptual	Structured through high-level dimensions, extensible dimensions	Emphasis technical categories, may not align with non-technical stakeholders interest, overwhelming when expanded
Mixed	Integration of other taxonomies, breadth and comprehensiveness, modularity to adopt other categories specific to a certain domain	Oversimplified overlaps by assuming all categories equally weighted, subjectivity on exclusion of dimensions



**Figure 1.** Classifying XML techniques using different taxonomies

incorrect predictions), and influential instances (which training data points had the most impact on the model's behavior). Concept-based taxonomies utilize various dimensions to construct concepts, including stage, applicability, and scope, for classifying XML approaches [40,52]. It is worth noting that additional dimensions exist in the literature for categorizing XML techniques [53,54]. In the context of structural

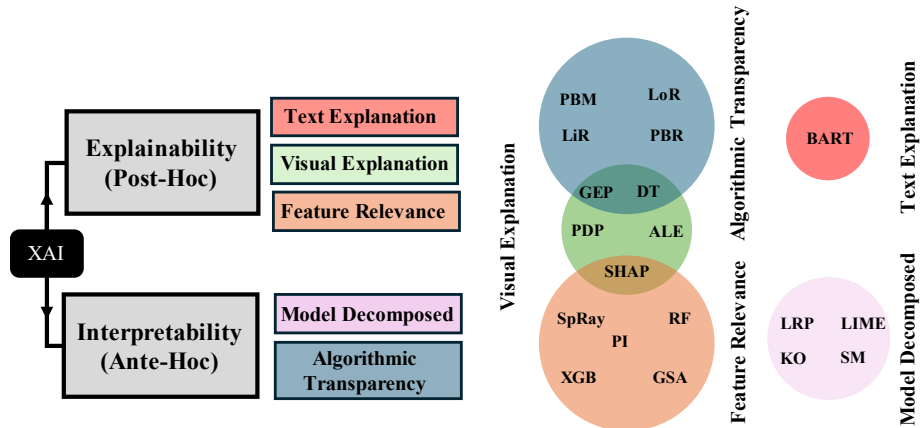


Figure 2. Common XML techniques

engineering, no effort has been made to develop such a domain-specific taxonomy. A mixed taxonomy (borrowing elements from other taxonomies; similar to work by Angelov et al. [41], Belle and Papantonis [14], and Minh et al.[8], among others) may better serve the diverse requirements of the field by adopting multiple dimensions, including ML-related (e.g., mechanism or context), engineering-related (physical interpretations or computational cost), and stakeholder-related (e.g., regulatory relevance) dimensions. In contrast, a result-based taxonomy offers the most straightforward adoption as it is intuitive (e.g., structural engineers are familiar with surrogate models and feature importance analysis) and requires less technical cost to implement. Developing such a taxonomy is a crucial step toward a more standardized use of XML in the field.

## 2.4. Description of common XML techniques

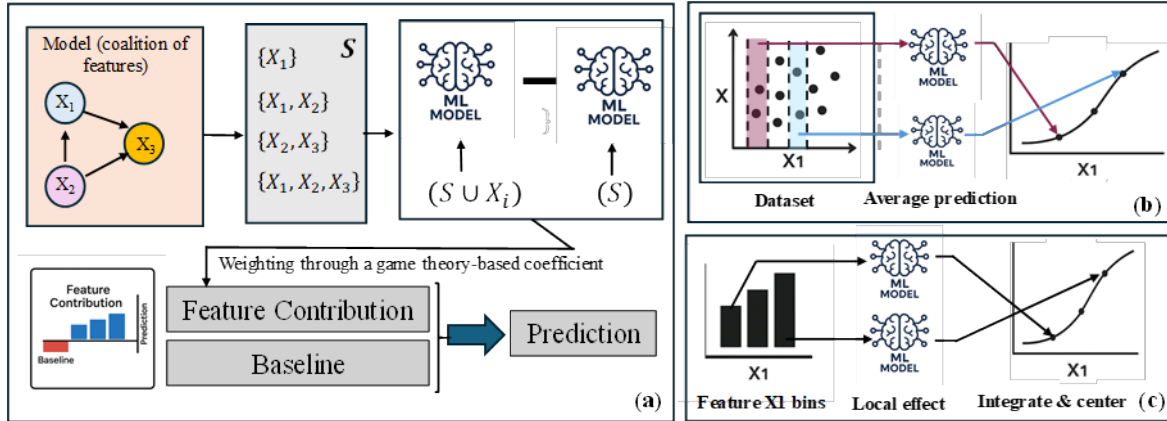
As discussed in the previous section, ML can be explained through two main avenues: post-hoc and ante-hoc (Figure 2). This section describes several widely used XML techniques in each category.

Three common post-hoc explainability methods are Shapley additive explanations (SHAP) [55], partial dependence plots (PDP) [56], and accumulated local effects (ALE) [57]. SHAP draws on game theory to assign each feature an importance value by treating inputs as players in a coalition game, where the model output is the shared payoff [41]. PDP and ALE visualize how individual inputs influence model predictions, capturing both linear and nonlinear relationships. While PDP measures the marginal effect of a feature on the average output, it ignores feature dependencies. ALE addresses this by using conditional distributions and averaging local differences, making it more robust to correlated inputs. Figure 3 illustrates the inner workings of these algorithms.

Two common methods of achieving XML through the form of the model (i.e., ante-hoc or white-box models) are genetic programming (GP) [58] and genetic expression programming (GEP) [59]. Both algorithms produce a model that is represented as a simple mathematical equation, allowing for the relationships between features to be easily explained when compared to more complex ML techniques such as artificial neural networks.

Physics-informed ML (PIML) models represent a form of ML models that possess elements of both ante-hoc and post-hoc XMLs: PIMLs embed physical laws directly into the ML architecture, which can be viewed as some level of built-in (algorithmic) explainability; however, they still leverage deep neural

networks that are complex and require post-hoc XML techniques to be fully understood. Common structural engineering applications, such as infrastructure performance assessment and reliability analysis, often rely



**Figure 3.** Schematics of how three widely used post-hoc XML techniques work: (a) Shapley explainers, (b) partial dependence plots, and (c) accumulated local effect

**Table 3.** Representative studies on using Physics-based ML techniques used in structural engineering

References	Application	Model Trained	Performance Evaluation Technique
[60]	Damage Classification	Regression Trees, LDA, KNN, DT, RF	Confusion Matrix
[61,62]	Structural Performance Prediction	DNN, LSTM	Mean Absolute Error, Root Mean Squared Error, Relative Error
[63,64]	Structural Health Monitoring	Gaussian Mixture Model, Physics-guided Neural Network	Type I, Type II Errors, Variance Variation
[65,66]	Pipelines Condition Assessment and Failure Analysis	KNN, DT, RF, NB, AdaBoost, LGBost, XGBost, CatBoost	Correlation Matrix, Confusion Matrix
[67]	Data Generation	RT, GAN	ROC, Confusion Matrix

on traditional physics-based models. However, these conventional physics-based models face computational challenges when dealing with complex and large-scale engineering problems [68,69]. PIML models integrate physical laws into ML models, enhancing the accuracy and explainability of predictions while overcoming computational challenges [68,70]. While ML models typically require input, output, and validation data for training, physics-based simulations can generate or augment these datasets [67]. Recently, generative adversarial networks (GANs) have gained traction in structural engineering for producing realistic synthetic data, particularly for failure analysis and structural health monitoring [67,71]. Table 3 shows various physics-based ML models, their applications, and performance evaluation techniques in structural engineering problems in recent years.

The explainability of PIML models depicts how physical laws are embedded into ML models and how information is transformed and exchanged within their algorithms. Since PIML models feed mechanical properties into the nodes/layers of the ML architecture and ground them on physical laws and principles, they are more interpretable than other ML architectures. Furthermore, these models and ML algorithms can be performed simultaneously, resulting in hybrid physics-informed ML models with higher accuracy [72].

Researchers also use knowledge-driven facts to interpret ML models with higher accuracy, referred to as Knowledge Ontology [73]. This process helps address confusion when a single symbol/character/icon is used for multiple semantic relationships [74]. Other techniques include the visualization of latent spaces, which analyze the internal relationships of models; physics-based regularization, which evaluates physical constraints during the learning process; gradient-based methods; and cross-validation with physics-informed metrics.

### **3. XML application in structural engineering**

XML has emerged as a powerful tool in structural engineering, enhancing prediction, assessment, and optimization across material, component, structural, and regional levels. To explore the state-of-the-art and practical applications of XML in structural engineering, this section reviews its implementation at the material and component levels, the structural level, and the regional level.

#### **3.1. XML for component-level and material-level assessment**

This section reviews the application of XML in component-level assessment, including behavior prediction, damage assessment, and design optimization, as well as material-level assessment, focusing on concrete, steel, and their associated bonds. Table 4 provides representative publications using XML for this domain. A general finding is that SHAP has been the most widely used XML technique for both component- and material-level assessment, covering almost 100% among the representative publications. PDP and ALE are employed in relatively few investigations (occupying about 13%) to reveal how strengths of material or components are affected by design parameters. Significant efforts have been made in component-level XML assessment, to explain the influence of material and structural design parameters on failure modes and component capacity properties. The examined capacity properties include plastic hinge lengths, compressive and shear strength, drift and curvature-based limit states of various types of components such as columns, walls, slabs, beams, joints, and pile foundations.

For columns, XML has been widely applied to classify failure modes and predict capacity properties. Mangalathu et al. [60] used XML to classify failure modes of reinforced concrete (RC) columns, while Wakjira et al. [75] predicted the plastic hinge length of rectangular RC columns. Wang et al. [76] used XML to predict the drift capacity of precast concrete columns, and Mansouri et al. [77] developed an XML model to estimate the shear strength of circular concrete-filled steel tubular columns. Moreover, XML models have been used to explain the compressive strengths or axial load-carrying capacity of various column types [78–83]. For walls, XML has also been applied to primarily predict shear strength and failure modes. Feng et al. [84] developed an XML model for shear strength prediction of squat RC walls, and Mangalathu et al. [60] used XML to classify failure modes of RC shear walls. Chen et al. [85] used XML for seismic damage evaluation of RC walls based on surface crack images, and Zhang et al. [86] further extended XML applications to predict the shear strength of RC flanged shear walls. For slabs, XML has been used to estimate punching shear strength, which is a critical factor in slab design. Wu and Zhou [87] applied XML to predict the punching shear strength of two-way reinforced concrete slabs, while Mangalathu et al. [88] focused on flat slabs without transverse reinforcement. For beams, Wakjira et al. [89] used XML to predict the shear strength of RC beams, while Zhang et al. [90] applied XML to estimate the flexural capacity of FRP-strengthened RC beams. Additionally, SHAP was used to explain the feature behavior of five ML models for the prediction of T-beam shear strength. These models included decision trees, which are themselves an XML model, along with random forest, GBDT, LightGBM, and XGBoost [91]. Through the use of SHAP, a wide range of ML techniques can become explainable, allowing for a

more widespread acceptance of these techniques in practice and in research. GP has also been used to model deep beam shear [92], steel fiber reinforced concrete beams [93], and beams without stirrups [94].

**Table 4.** Representative publications focusing on XML applied to structural material and component-level ML studies.

References	Studied component/material	Target Variable	Best ML model employed <sup>a</sup>	Explainability technique(s)
<b>Structural Material-level Assessment</b>				
[95]	Concrete	Compressive strength	LightGBM	SHAP
[96]	Concrete with manufactured sand	Compressive and tensile strength	XGBoost	SHAP
[62]	Steel fiber-reinforced recycled aggregate concrete	Compressive and splitting tensile strength	GBRT and AdaBoost	SHAP, PDP
[97]	Concrete	Creep behavior	LGBM	SHAP
[98]	Corroded reinforced concrete	Bond strength of corroded reinforcement	Combination of KNN, MLP, RF and XGBoost	SHAP
[99]	Ultra-high-performance concrete and normal concrete interface	Bond strength	CatBoost	SHAP
Ke et al. 2023b	CFRP-steel epoxy-bonded interfaces	CFRP-steel interface bond strength	CatBoost	SHAP
[101]	Steel	Fatigue strength	Hybrid model combining XGBoost and LightGBM	SHAP
Kulasooriya et al. 2023	Basalt-fiber reinforced concrete (BFRC)	Compressive, flexural, and tensile strength	Decision Tree, Gradient Boosting Tree, LightGBM	SHAP, LIME
<b>Structural Component-level Assessment</b>				
[60]	RC column	Failure mode classification	RF classifier	SHAP
[75]	RC column	Capacity prediction (drift, axial strength)	Stacking ensemble model	SHAP
[76]	Precast concrete columns	Drift capacity	XGBoost	SHAP
[102]	Bridge piers	Seismic demand and fragility	ANN	SHAP
[77]	Circular concrete-filled steel tubes	Shear strength	XGBoost	SHAP
[103]	Timber column	Fire resistance	RF	SHAP, PDP
[84]	Wall	Shear strength	XGBoost	SHAP
[86]	Wall	Shear strength	XGBoost	SHAP
[104]	Wall	Failure modes	XGBoost	PDP, ALE, SHAP
[86]	Wall	Damage and crack pattern	NN, RF, DT	SHAP

[105]	Wall	The ration of Shear wall area-to-floor area	GBDT	SHAP
[87]	Slab	Punching shear strength	PSO-SVR	SHAP
[88]	Slab	Punching shear strength	XGBoost	SHAP
[89]	Beam	Shear and flexural strength	XGBoost	SHAP
[90]	Beam	Flexural capacity	GBDT	SHAP
[106]	Connection	Failure mode	XGBoost	SHAP
[107]	Connection	Shear capacity	GBM	SHAP
[108]	Connection	Fatigue failure of butt-welded connections	XGBoost	SHAP

For joints, Gao and Lin [106] developed an XML model to predict the failure mode of beam-column joints, while Ye et al. 2024 used XML to estimate the shear capacity of UHPC joints. In addition, Barun et al. [108] developed an XML model to predict fatigue failure in small-scale butt-welded joints. For pile foundations, XML has been used to predict load-bearing capacity and seismic energy dissipation mechanisms. Karakaş et al. [109] applied XML to predict the ultimate bearing capacity of piles, while Wang et al. [110] used XML to assess seismic energy dissipation mechanisms in scoured bridge pile-group foundations. Lastly, XML has used for ML beyond conventional structural components, for example, to provide explainability to a model for wind turbine blade damage [111].

A limited number of component-level studies focused on optimizing component design. Reinforcement layouts in RC columns and beams have been optimized using XML to ensure both performance and cost-effectiveness. For example, Cakiroglu et al. [112] applied XML to optimize the pile diameter for cantilever soldier pile retaining walls, while Iqbal et al. [113] used XML to study the effects of rebar size and volume fraction of glass fibers on the tensile strength retention of GFRP rebars in alkaline environments.

For material-level assessment, XML has been applied to predict the compressive strength, elastic modulus, and bond strength of various types of concrete. Jia et al. [95] and Lyngdoh et al. [96] used XML to predict the compressive strength of concrete, while Zhang et al. [62] applied XML to estimate the compressive and splitting tensile strength of steel fiber-reinforced recycled aggregate concrete. Liang et al. [97] developed an XML model to predict the creep behavior of concrete, and Hu et al. [99] used XML to estimate the bond strength between UHPC and normal-strength concrete. SHAP was further used to provide explainability for eight machine learning models including random forest, support vector machine, and artificial neural networks for concrete strength predictions [114]. Through the use of SHAP the authors were able to determine not only the impacts of the features on the final prediction but also on each other. Focusing on ante-hoc explainability, GEP has been used to predict the compressive strength of high-strength concrete [115], bagasse ash-based concrete [116], and geopolymers concrete [117]. In these studies, the GEP algorithm was utilized to generate a model that is highly explainable, allowing for further analysis and commentary on the relationships between the features. Beyond concrete, XML has also been used to predict the bond strength and fatigue life of steel. Ke et al. [100] developed an XML model to predict the bond strength of CFRP-steel interfaces, while Yan et al. [101] applied XML to predict the fatigue strength of steel. Ge et al. [98] used XML to predict the bond strength of corroded reinforced concrete following high-temperature exposure.

### 3.2. XML for structure-level design

The application of XML techniques in building design and assessment has gained significant attention over the past five years (Table 5), particularly within the framework of performance-based earthquake engineering (PBEE) for explaining the roles of structural and seismic intensity parameters in seismic response, capacity, fragility, loss, and resilience. SHAP plays the dominant role among all the XML

**Table 5.** Representative publications focusing on XML applied to structural-level ML studies

<b>Publication</b>	<b>Structure</b>	<b>Target Variable</b>	<b>Best ML model employed</b>	<b>Explainability technique(s)</b>
[118]	Masonry-infilled frames	Natural vibration period of masonry infill concrete frame structures	XGBoost	SA, SHAP, ICE
[119]	Masonry-infilled frames	Fundamental period of vibration of masonry infilled RC frames	NN	PDP, ICE, ALE, SHAP
[120]	Masonry-infilled frames	Fundamental time period of masonry-infilled RC frames	XGBoost	SHAP
[121]	Masonry-infilled frames	Natural period of vibration	RF	SHAP, LIME, PDP
[122]	Steel moment frames	Seismic damage states	RF	SHAP
[123]	Steel moment frames	Fragility function parameters	ANN	SHAP
[124]	RC frame buildings	Seismic fragility curve parameters	XGBoost	SHAP
[125]	RC frame buildings	Compressive membrane/arch action resistance, Progressive collapse resistance-displacement curve	DCN	SHAP
[126]	Seismically deficient RC frame	Damage index	LGBM	SHAP, PDP, LIME, ALE
[127]	High-rise building with interference from a low-rise building	Wind pressure coefficients	XGBoost	SHAP
[128]	Bridges	Pier drift ratios, Bearing displacements, Deck displacements	XGBoost with Bayesian optimization	SHAP
[129]	Bridges	Damage classification	ANN, MLP and RF	SHAP

[130]	Bridges	Repair cost of earthquake-damaged bridges	Extra Trees ensemble	SHAP, Feature dependence plots
[111]	wind turbine blade	Mahalanobis distance-based damage index	XGBoost	SHAP
[131]	pipelines	Failure modes of corroded pipelines	XGBoost	SHAP
[132]	Steel Moment-Resisting Frames	Resilience index	XGBoost	SHAP
[133]	Bare frames, and frames with infills	Maximum inter-story drift ratio	GPR	SHAP
[134]	RC buildings	Maximum Inter-story Drift Ratio (MIDR)	LightGBM	Contributions Plots, PDP, Shapley values, SHAP
[135]	Steel moment frames	Peak inter-story drift ratio (PIDR), Peak floor acceleration (PFA)	GPR	SHAP
[136]	Cross-Laminated Timber (CLT) buildings	Maximum Inter-story Drift Ratio , Maximum Roof Drift Ratio	RF	SHAP
[137]	Steel frames equipped with shape memory alloys (SMAs)	Amplification coefficient for energy modification factor	XGBoost	PDP, SHAP
[138]	High-rise buildings with outrigger systems	Maximum inter-story drift, Maximum top acceleration, Maximum top displacement	DNNs	PDP, SHAP
[139]	Construction projects	Cost impacts of nonconformances	RF algorithm with Gravitational Search Algorithm	SHAP
[140]	RC and masonry buildings	Seismic vulnerability index	ANN	SHAP
[141]	Buildings	Overall damage grade, Building collapse grade and Building leaning grade	XGBoost with SMOTE	SHAP
[142]	Two-story damped steel frame	Maximum inter-story displacement	Aggregation model combining XGBoost and SWT	SHAP
[143]	High-rise buildings	Unit construction cost	BP-SVM combined algorithm	SHAP, PDP, and ICE
[144]	High-rise buildings	Aerodynamic force coefficients, and Wind-induced dynamic response coefficients	XGBoost	SHAP

[145]	Residential buildings	Fire damage area and Fatality/Casualty prediction	XGBoost	SHAP
[146]	Buildings	Property loss rate due	IBTEM	SHAP, PDP
[147]	Offshore concrete bridge	Life-cycle seismic performance	LSTM	LSTM-Shapley framework
[148]	Buildings	Seismic performance metric	XGBoost	SHAP, LIME, PDP, ICE
[149]	Tunnel	Tunnel damage state classification, Damage indicators, and Seismic fragility prediction	XGBoost	SHAP, and Feature importance score analysis
[150]	RC Bridges	Post-earthquake vertical load-carrying capacity	ANN	SHAP,PDP

techniques (occupying over 95%), while about 25% studies adopt ICE, PDP, or LIME for structural assessment and analyses of frames and buildings.

The studies include: (1) Identifying Critical Parameters: using techniques such as SHAP, ICE, LIME, or PDP to identify critical structure-related parameters that significantly affect natural (fundamental) periods of masonry-infilled frames [118–121]; (2) Explaining Seismic Response Influences: leveraging XML to interpret how seismic responses (e.g., maximum inter-story drifts and floor acceleration) of buildings are effected by structural and seismic loading features [133–138,142]; (3) Seismic Capacity and Damage Analysis: utilizing SHAP to investigate the impact of various features on seismic capacity and damage states of steel moment frames [122]; (4) **Fragility**: employing SHAP to identify key parameters in predicting the seismic fragility of steel moment-, reinforced concrete-, and masonry-frame buildings [123,124,140,141]; (5) **Loss Estimation**: applying PDP to explore the effects of structural-modeling-related parameters on seismic loss of steel moment-frame buildings [151–154]; and (6) **Resilience Assessment**: using SHAP to analyze the importance, effects, and interactions of design parameters in evaluating seismic resilience of buildings [132]. Beyond earthquake applications, XML techniques have also been adopted to explain the developed ML models for assessing construction quality, predicting costs [139,143], estimating progressive collapse resistance [125], evaluating aerodynamic interference effects between tall and short buildings [127,144], and fire loss prediction [145,146].

For bridges, a critical transportation infrastructure, XML techniques have been implemented to understand how design features affect behaviors of bridges under operational conditions or against natural hazards, especially within the PBEE framework. Applications include (1) **Seismic Response Prediction**: Evaluation of seismic responses in terms of the column drift, bearing deformation and superstructure acceleration incorporating time-dependent material deterioration and multiple sources of uncertainty [128,147,155]; (2) **Damage State Classification**: classification of seismic damage state using numerical models considering aging corrosion effect or using the post-earthquake damage data [129,156]; (3) **Seismic Fragility Assessment**: application of XML techniques to assess the fragility of rocking column-supported bridges [157]; and (4) **Repair Cost Predictions**: prediction of post-earthquake repair cost of damaged bridges [130]. Despite these advancements, applications of XML in seismic resilience assessment for

bridges remain unexplored as of the time of this manuscript's preparation. In addition to earthquakes, XML have also been applied to bridge health monitoring. using XML techniques and structural health monitoring (SHM) data, researchers have predicted and interpreted load-deformation correlation and cumulative displacement of long-span suspension bridges subjected to temperature, wind and vehicle loads [158–160].

Besides building and bridge structures, XML techniques have been extended to various other areas in structural engineering, including nuclear power plants [161], wind turbines [111], pipelines [131], road embankments [162], tunnels [149], deep beam shear estimation [163], and gridshell structures [164].

### **3.4. XML For regional-level assessment**

ML vulnerability and risk assessment models are increasingly being developed for infrastructure systems and buildings and bridges at regional scales. These models address some of the limitations of traditional analytical and empirical approaches by reducing computational costs for regional seismic loss estimation, handling numerous input variables and complex spatial dependencies in infrastructure assessments, and capturing intricate interrelationships among physical, social, and economic factors necessary for community resilience modeling [165,166]. Since ML vulnerability models for infrastructure systems and regional assessments are intended to support mitigation and recovery planning, it is crucial to understand the predictions provided by these models. However, the lack of explainability and interpretability in ML models hinders such understanding. To address this issue, efforts have been made to interpret and explain ML models for regional and system-level assessments. The most common explainability techniques used in the literature for infrastructure systems and regional assessments are PDP, ALE, and SHAP, as summarized in Table 6.

Several studies have employed PDP and ALE to explain ML models for buildings, bridges, and infrastructure systems. For example, [167] used PDP to explain an ML model developed to predict climate-induced disaster-related damages, focusing on property damage caused by wind hazards. The study considered attributes related to hazard, climate, land cover, social, housing, demographic and economic data. Various ML techniques were evaluated, and random forest-based regression tree was found to be the best performing model. PDP was successfully used to explain the complex interrelationships between input variables and predicted property damage. The impact of each individual variable and pairs of variables was studied. Johnson et al. [166] used PDPs as a tool to depict spatial network vulnerabilities to disruptions. A GBM model was trained to analyze disruptions in a freight network. The PDPs were then used to study how disruptions at different locations impacted network efficiency while controlling for other input variables and spatial dependencies. In a similar approach, [168] used ALE as a tool to examine the relationship between road network attributes and groundwater quality parameters, aiming to identify road transportation factors influencing water quality.

SHAP is the most common technique for explaining ML models for infrastructure systems and building and bridge inventories. [169] used SHAP to explain the results of an extreme gradient boosting model developed to predict and analyze electricity, gas, and water consumption in large-scale buildings in New York City. Similarly, Chen and Zhang [170] and Roeslin et al. [171] also employed SHAP to explain ML models for regional seismic risk and damage assessment.

Mangalathu et al. [104] compared the use of PDP, ALE, and SHAP to explain ML models for regional-scale bridge damage assessment in California. Seven ML models were evaluated and XGBoost was found to be the best performing model. XGBoost's importance factor was also considered as an explainability technique. The authors compared the four methods in terms of their ability to rank input variables, identify trends, predict influence on output, explain specific predictions, and explain how the model makes decision.

XGBoost importance factor can only rank input variables and identify any correlations between them. PDP and ALE offer similar insights, but PDP cannot capture correlations between input variables. SHAP is the only technique that can explain the risk predictions and identify the factors that contribute to the predictions. The authors concluded that no single technique could fully explain the ML models and recommended the use of more than one technique to adequately understand and explain ML models.

**Table 6.** Representative publications focusing on XML applied to regional-level ML studies

Ref	Studied infrastructure	ML models employed	Explainability technique(s)
[104]	Regional damage assessment of bridges in California.	7 models (Ridge, SVR, KNN, DT, RF, AdaBoost, XGBoost)	ALE, PDP, SHAP
[168]	Impact of road transport systems on groundwater quality.	XGBoost	ALE
[166]	Freight network vulnerabilities (intermodal road and rail freight transportation network).	GBM	PDP
[169]	Interrelationships between electricity, gas, and water consumption in large-scale buildings.	XGBoost	SHAP
[167]	Predict wind-related property damages based on disaster, landcover, climate, social, economic, housing, and demographic-related records.	Classic regression trees and regression trees ensemble techniques (i.e., bagging, random forest, and boosting)	PDP
[172]	Assess regional seismic resilience considering geological, building, and social dimensions.	Dempster-Shafer evidence theory and cluster analysis	Global sensitivity analysis
[170]	An automated machine learning (AutoML) framework to predict the casualty rate and direct economic loss induced by earthquakes.	SVM, kNN, RF, XGBoost, CatBoost, and LightGBM	SHAP
[171]	Building damage prediction model using empirical regional data.	Logistic regression, SVM, DT, and RF	SHAP
[173]	Improving the seismic performance of a regional group of small-to-medium span highway bridges.	ANN	SHAP
[174]	Seismic repair cost rate prediction for residential buildings due to seismic damage.	Three distance-based regression models, a Decision Tree Regression model and two Ensemble regression models (RF and Gradient Boosting Regression).	SHAP

#### 4. Illustrative Examples

This section presents three case studies that apply different XML techniques to various structural engineering problems at three different scales: component-, structure-, and inventory-level to showcase the potential of these approaches over a diverse set of problems. The first example uses an ante-hoc XML technique based on genetic expression programming, whereas the other two models use post-hoc explainability based on SHAP, permutation importance, and accumulated local effects.

## **4.1. XML for component level assessment: Ultimate shear prediction of deep reinforced concrete beams with knowledge transfer from slender RC beams**

The first illustrative example investigates the application of XML to the problem of modeling deep beam shear strength through the use of genetic expression programming (GEP) [175] and transfer learning (TL) [176,177].

### **4.1.1. Dataset and Background**

In structural engineering, collecting high-quality experimental data is often constrained by cost, time, and practical feasibility. To address this issue, this study explores data-scarce modeling through XML, with an emphasis on model explainability, which is crucial for engineering judgment and design decision-making. Transfer learning is used here to improve predictions on a small “target” dataset by leveraging knowledge from a larger, related “source” dataset. TL is a more complex task than traditional ML as it involves multiple datasets and mechanisms for transferring knowledge from the source to the target in a beneficial manner. Genetic expressions programming (GEP) is used to address the issue of explainability as it is a white-box model where the relationship between features used to make predictions are plainly observable.

The dataset utilized in this study consists of 1,849 data points spanning a wide range of deep and slender reinforced concrete beams without shear reinforcement [178]. This dataset consists of material properties of the beam’s constituent materials as well as the geometric properties of the beam itself. The data was collected over the span of 59 years from 1948 to 2007. The beams contained within the dataset are both high- and low-strength concrete, square and circular cross sections, and tested under various loading configurations. The slender beams, those having an  $a/d$  ratio of more than 2.5, were taken as the source dataset to pretrain the model and the deep beams, those having  $a/d$  ratio less than 2.5, were used as the target dataset and used to fine tune the model. The dataset features can be seen in Table 7. The data is log scaled before being used as input for training and prediction.

### **4.1.2. Model Development**

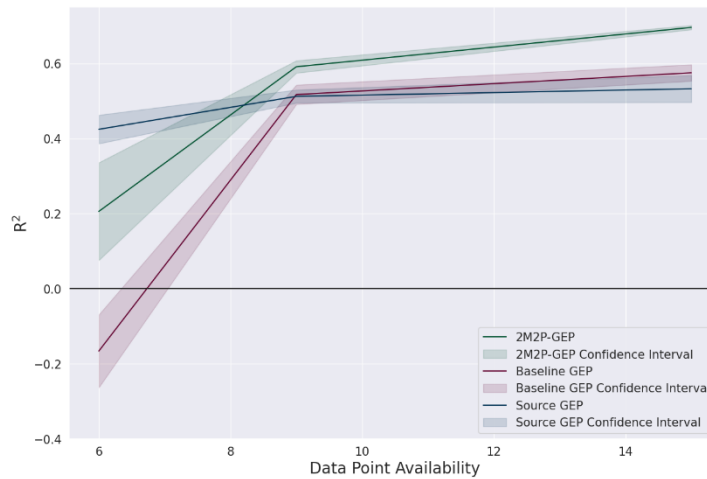
To accommodate differences between source and target datasets, the 2M2P algorithm (Figure 4) is employed. This method is designed for both homogeneous (same features) and heterogeneous (different features) transfer scenarios. To address the feature space mismatch zero vectors are added for missing features, creating a shared input space. A GEP population is first trained on the source dataset. This trained population is then evaluated on the target data, and the best-performing individuals are selected. These models undergo a forced mutation, a step that introduces small random changes to adapt the model to the target data while preserving learned knowledge. The mutated population is then fine-tuned on the target training set, and the best model is selected based on validation performance. Table 8 provides the hyperparameters utilized in model training.

In this application the source data consists of slender beam shear strength tests, while the target data consists of deep beam shear strength tests. To mimic real-world data scarcity, only 6, 9, and 15 deep beam data points are used. Furthermore, certain features are intentionally removed to simulate heterogeneous transfer conditions. Three models are compared: A source-only GEP model trained only on the slender beam data, a baseline GEP model trained only on the few deep beam data points, and the 2M2P model incorporating transfer learning.

The results can be seen in Figure 4. When only 6 deep beam points are available, the source model performs best. However, at 9 and 15 data points, the 2M2P model consistently outperforms both the source and baseline models, demonstrating stable performance across 10 randomized runs. The best-

**Table 7.** RC Beam Dataset Features

Parameter	Description	Unit	Mean	Standard Deviation	Minimum	Maximum
$b_w$	Web Width	mm	213.01	212.94	21.00	3000.00
$b$	Flange Width	mm	256.76	230.21	21.00	3000.00
$h$	Depth	mm	364.35	254.05	51.00	3140.00
$d$	Effective Depth	mm	320.25	237.77	41.00	3000.00
$amv$	Distance from point of inflection to maximum moment location	mm	953.22	823.70	80.00	9000.00
$astm$	Distance from center of applied load to the center of support	mm	1000.97	845.97	80.00	9000.00
$a/d$	Span to depth ratio	-	3.20	1.82	0.25	15.06
$bear$	Length of bearing pole	mm	107.13	75.15	0.00	6000.00
$\rho_w$	Horizontal reinforcement ratio	%	2.24	1.52	0.10	9.50
$f'_c$	Concrete compressive strength	MPa	34.86	18.34	6.10	127.50
$ag$	Maximum aggregate size	mm	18.48	6.96	1.00	50.00
$f_y$	Steel yield strength	MPa	462.37	172.14	267.00	1779.00
$mpvd$	Derived parameter	-	138.62	156.01	0.00	2768.30
$v_u$	Ultimate shear strength	MPa	129.59	153.21	1.90	1575.00



**Figure 4.** Data availability analysis

**Table 8.** Hyperparameters used for training ML of RC Beam example

<b>Parameter</b>	<b>Description</b>	<b>Values</b>
Head length	Number of operators and features in the head of each gene	3,5,8
Genes	Number of genes that comprise the chromosome	3,4,5,6
Population size	Number of chromosomes/expression trees in the population	36, <b>50</b>
Batches	Number of populations isolated from each other and evaluated	<b>50</b>
Source generations	Number of generations the source model is allowed to execute	<b>200</b>
Baseline generations	Number of generations the target model is allowed to execute	<b>400</b>
Transfer generations	Number of generations the transfer model is allowed to execute	<b>200</b>
Tournament size	Number of chromosomes/expression trees competing with each other for reproduction	3,5,10,30
Champions	Number of highest scoring chromosomes/expression trees copied from previous generation	<b>3</b>
Linking function	Function that links the chromosomes	addition, <b>multiplication</b>
Fitness function	Function utilized to evaluate the fitness of the chromosomes/expression trees	<b>MAE</b>
Transfer Function	Function used to select individuals for transfer	<b>SSIP, WPE</b>
Operators	Mathematical operations used in the chromosomes/expression trees	<b>addition, subtraction, multiplication, division</b>

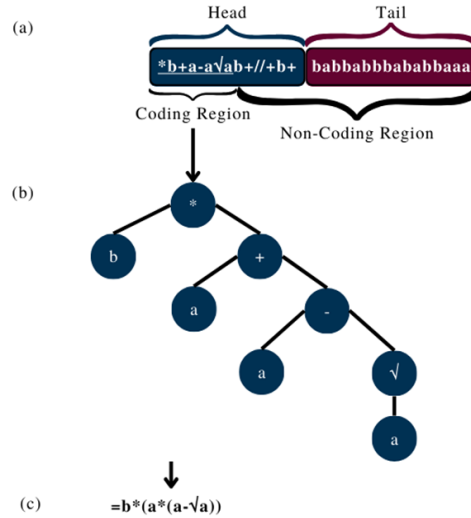
performing model generated by the 2M2P algorithm for the 15 data point availability can be seen below. Equation 1 illustrates the high level of interpretability as the equation is simply the product of three terms consisting of variables that represent values commonly collected during beam experiments.

$$V_u = \left( b_w * (-astm + d) + d * \sqrt{f'_c} \right) * \left( -\frac{a}{d} + b + f'_c * (b_w - 5) \right) \quad (1)$$

$$(b + f'_c * (b_w - 5) - h)$$

#### 4.1.2. Explainability

The explainability of this XML approach is rooted in the architecture of the GEP model. As a white-box method, GEP provides full transparency into the steps that link input features to the model's predictions. The model comprises two interconnected components: the genotype and the phenotype (Ferreira 2001). The genotype, which is shaped during the training and mutation processes, is a linear string of characters that includes both operators, such as mathematical functions like addition or multiplication, and terminals, which represent constants or dataset variables. Additionally, the genotype maintains a modular organization



**Figure 5.** GEP model form (a) the genotype string (b) the phenotype expression tree (c) the

in which the "head" section contains the active, expressed portion of the string, while the "tail" consists of unused elements reserved for potential structural expansion. During the prediction phase, this string is decoded by the GEP algorithm and expressed as a mathematical equation, known as the phenotype, which is used in prediction and evaluation. This transformation reveals the internal logic of the model in a form that is both readable and interpretable. The resulting equation can be viewed directly, as shown in Figure 5(c), or visualized as an expression tree, as illustrated in Figure 5(b), offering multiple layers of interpretability. Because the final model is a concise mathematical expression composed of familiar engineering variables, its structure can be readily understood by domain experts. This design ensures that the model remains both flexible and interpretable, making it particularly well-suited for use in civil engineering applications where understanding the relationship between variables is essential.

## 4.2. XML for structure-level assessment: Evaluating the impact of structural parameters and seismic intensity measures on seismic responses of multi-span highway girder bridges

The second illustrative example explores the capability of XML in interpreting a traditional Neural Network (NN) trained to predict longitudinal seismic responses of regular multi-span highway girder bridges. The database was developed using finite element modeling accounting for various parameters of the bridges.

### 4.2.1. Database and model features

The examined bridges are ordinary highway bridges among the national highway systems in China. They have a typical span length of 30 m to adapt to industrial prefabricated superstructures, which are supported by reinforced concrete (RC) bridge bents and abutments through spherical steel bearings. One of the bents has longitudinally fixed bearings while others have sliding bearings. Each bent comprises a cap beam, two extended pile-shafts (i.e., Type II with capacity-protected pile). For this reason, the bottom of columns is fixed in the three-dimensional finite element models established in OpenSees [179], where nonlinear fiber-section-based beam-column elements and bilinear force-displacement constitutive models are assigned to columns and sliding bearings, respectively, to capture their nonlinear behavior under earthquakes. For conciseness, detailed modeling techniques can be referred to [180].

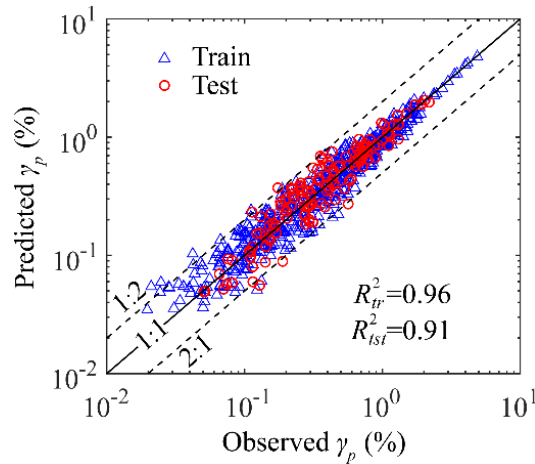
To develop a sound database, five parameters of bridges were considered as uniformly distributed random variables and selected as features (input variables) of the NN model: column diameter ( $D \sim U[1.4m,$

2.5m]), column axial load ratio ( $\alpha \sim U[0.1, 0.3]$ ), column height ( $H \sim U[10\text{m}, 25\text{m}]$ ), column longitudinal reinforcement ratio ( $\rho_l \sim U[0.6\%, 2.5\%]$ ), and number of span ( $N_s \sim U[2, 5]$ ). Note that the assumed uniform distributions aim to cover a wide range of engineering scenarios as well as to assure a balanced database for the expediency of model training. Accordingly, 720 bridges were sampled using Latin hypercube sampling, and randomly paired to a selected 720 ground motions (Wang et al. 2021; Baker et al. 2011) for a total number of 720 nonlinear time-history analyses. The peak column drift ( $\gamma_p$ ) is adopted as the engineering demand parameter for seismic performance assessment, and thereby chosen as the label (output variable) of the NN model. On the other hand, multiple seismic intensity features are often required to achieve high-confidence ML modeling of structures [182]. Accordingly, seven seismic intensity measures were also selected as features of the NN model: peak ground acceleration (*PGA*), peak ground velocity (*PGV*), peak ground displacement (*PGD*), spectral acceleration at a period of 1 second (*Sa10*), spectral velocity at a period of 1 second (*Sv10*), Housner intensity (*HI*), and the state-of-the-art average spectral acceleration (*AvgSa*) with proper lower and upper period bonds for highway bridges (Feng et al., 2024). Finally, a comprehensive dataset consisting of 720 data for 12 features (including five structural parameters and seven seismic intensity measures) and the label (peak column drift) was established and can be accessed via Zonodo (<https://doi.org/10.5281/zenodo.14211631>).

#### 4.2.2. Model development

The dataset was normalized using the Z-score method and then randomly grouped into a training set and a test set by 80% and 20%, respectively. Next, a traditional NN model was trained using the training data and an open-source Scikit-learn package in Python. Except for hidden layer sizes, other hyperparameters of the NN model were taken as the optional values provided by Scikit-learn. To reduce model complexity (avoiding overfitting) and minimize the computational demand for hyperparameter optimization, both the number of hidden layer  $N$  and the number of neurons in each hidden layer  $Q$  were restricted to values less than 20. Using the grid search method,  $N$  and  $Q$  were tuned to be  $N = 11$  and  $Q = 6$  as the optimal NN hyperparameters. This optimization process took approximately 1,000 seconds on a personal computer with Intel Core i7 12700K @ 4.9 GHz processor and 32 GB DDR4 RAM.

Figure 6 shows the performance of the NN model on training and testing set. In generally, NN reasonably estimated peak column drifts with a R-squared value larger than 0.90. The average errors of NN model are



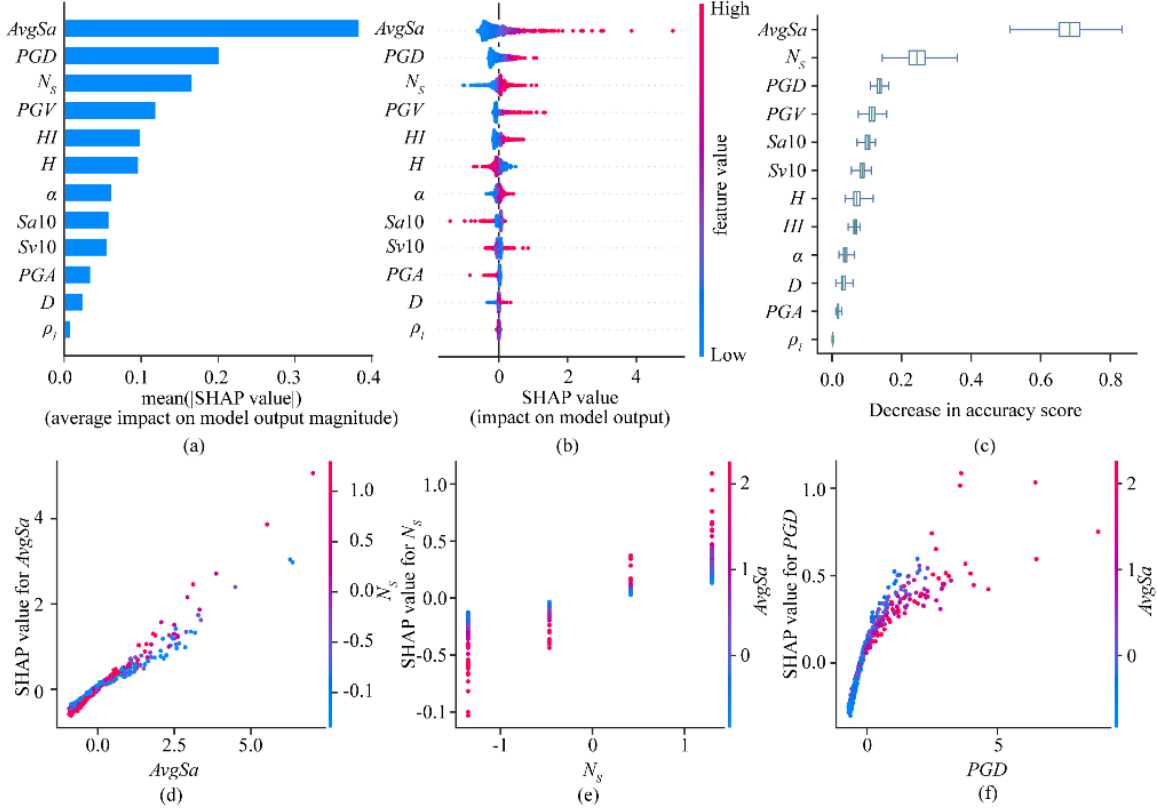
**Figure 6.** NN performance on training and testing set to predict peak column drifts

22.9 % and 23.7% on average for the training set and test set, respectively, falling into the typical error range from 15% to 25% for the applications of traditional ML in structural seismic response predictions [184,185]. For large drift responses (exceeding 1.0%) of primary concern, the prediction error decreases significantly to 9.6%. Therefore, the prediction accuracy of the ML model is generally acceptable.

#### 4.2.3. Explainability techniques

Two XML techniques, i.e., SHAP and permutation importance [186], were selected to explain the NN model. Figure 7(a) displays the ranking of feature importance, which is represented by the mean absolute value of feature SHAP values ( $\text{mean}(|\text{SHAP value}|)$ ) over all the samples (Mangalathu et al. 2020). Note that SHAP values presented in this example quantify the impact of features on the Z-score normalized peak column drifts. A larger mean ( $|\text{SHAP value}|$ ) indicates a higher importance. Overall, the seismic intensity measures play a more critical role than the structure-related ones in predicting peak column drifts. In particular, the state-of-the-art *AvgSa* is the most important feature with the largest mean ( $|\text{SHAP value}|$ ) of about 0.4, which indicates that a loss of *AvgSa* as the feature may lead to a change of peak column drift as large as approximately 40% of its standard deviation on average. Therefore, it is reasonable to adopt *AvgSa* as an optimal intensity measure for probabilistic seismic demand modeling of the investigated bridges in terms of peak column drifts for future studies. Figure 7(b) summarizes SHAP results over all the samples, where features are ranked in descending order of importance (i.e.,  $\text{mean}(|\text{SHAP}|)$ ) from top to bottom. Each point in the plot represents the feature SHAP value in an instance and is colored based on the feature value from blue (low) to red (high). When red dots cluster on the right (positive SHAP values) and blue dots cluster on the left (negative SHAP values), it indicates a positive correlation between the feature and the model output. Conversely, it indicates a negative correlation. It can be found from Figure 7(b) that red dots of seismic intensity-based features (particularly *AvgSa*, *PGD*, *PGV*, *HI*), number of spans ( $N_s$ ), and column axial load ratio ( $\alpha$ ) are mostly distributed on the right side of the figure with positive SHAP values. This implies a larger value of these features would trigger a larger peak column drift. By contrast, red dots of the column height ( $H$ ) are mostly negative, indicating that a higher column could lead to a smaller column response. Compared to the above features significant impact on peak column drifts. Specifically, the reinforcement ratio mainly influences the cases where the yielding of steel reinforcement occurs. The column diameter affects both the mass (i.e., the section area multiplied by the concrete strength and axial load ratio) and stiffness of structures, eventually exerting a limited impact on seismic responses.

To understand the stability of explainable results across different XML approaches for the examined bridges, Figure 7(c) shows the ranking of permutation-based feature importance, which is obtained by permuting the feature values and comparing the model loss with the permuted feature values to that with the original feature values [186]. A large decrease in model accuracy with the permuted feature values indicates a high importance. Note that each feature is permuted 1,000 times to obtain the statistical properties of permutation importance. In this box chart, three vertical lines of boxes from the left to right represent the 25% quantile, median, and 75% quantile of permutation importance, respectively. The feature importance ranking is determined based on the median permutation importance, while the confidence level of such a ranking can be evaluated by examining the variation of permutation importance. In general, the permutation-based importance ranking is quite close to the SHAP-based one in terms of the top three and last three influential features. From Figure 7(c), *AvgSa* is again identified as the most influential feature, i.e., permuting values of *AvgSa* would lead to most decrease in accuracy score of the NN model. By contrast, disrupting the sequence of column reinforcement ratio ( $\rho_l$ ) hardly lowers the accuracy of the model, indicating a negligible feature impact. It is worth noting that the span number ( $N_s$ ) exhibits a larger



**Figure 7.** Interpretation results for bridge seismic response predictions: (a) ranking of SHAP-based feature importance, (b) summary of SHAP values, (c) ranking of permutation-based feature importance, SHAP dependence plots for (d) AvgSa, (e)  $N_s$ , and (f) PGD.

permutation importance than *PGD* (see Figure 7(c)), whereas *PGD* is identified as more influential by SHAP (see Figure 7(b)). These results imply that applying various XML techniques helps to comprehensively explain the ML models. Nevertheless, the general trend of variable relative importance is quite stable for the examined case. Additionally, the permutation-based feature importance ranking exhibits relatively high credibility and stability, supported by minimal overlaps in the box plots of adjacent features.

To explore feature interactions, Figures 7 (d)-(f) present the SHAP dependence plots for top three influential features (i.e., *AvgSa*,  $N_s$ , and *PGD*). The horizontal and vertical axes represent the feature value and SHAP value, respectively, of the first feature. The color of point indicates the value of the second feature, with blue indicating low values and red indicating high values. From Figure 7(d), the dispersion of SHAP value for a given *AvgSa* value is relatively low, which means the effect of *AvgSa* on column drift estimates is less affected by other features. By contrast, *AvgSa* significantly influences the SHAP values of  $N_s$  and *PGD* (Figures 7(e) and (f)), which means the contributions of  $N_s$  and *PGD* on column drift predictions depend on the average spectral acceleration of ground motions. The results of feature interactions further highlight the finding that *AvgSa* exhibits an importance much higher than that of other features.

### **4.3. XML for inventory-level assessment: Evaluating the impact of design and geometry parameters on annualized loss of a steel building inventory**

The third illustrative example applies XML to understand an ML model trained to predict the seismic losses of a building inventory of 621 special moment-resisting frames.

#### **4.3.1. Database and model features**

The building inventory was generated through an automated seismic design tool, and covers a wide range of geometries (from 1- to 20-story buildings) with varying footprints and designs [188]. The building had a typical story height of 13 ft, with the first story height between 13 to 26ft. All buildings are assumed to be located at Los Angeles, California. Two-dimensional finite element models were generated in OpenSees, where frame elements were modeled using elastic elements with plastic hinges at both ends. The plastic hinges followed the Ibarra-Medina-Krawinkler model with bilinear hysteretic [189].

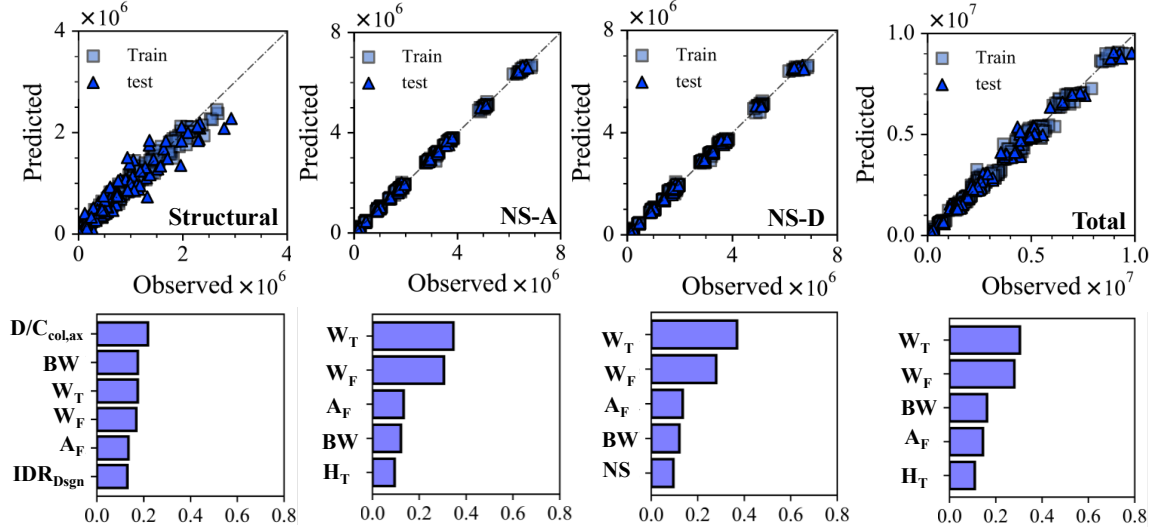
The buildings were subjected to 240 ground motions from 12 past events in California with a magnitude between 6 to 7, and peak ground acceleration of 0.03g to 0.61g [190]. The maximum drift and acceleration responses of all floors were estimated through dynamic nonlinear history analysis and post-processed to derive building-level fragility function and loss values. A component-based approach based on FEMA P-58 [191] was used for loss functions, where floor-level responses were used to quantify damage to structural and nonstructural components. It should be noted that the underlying data and assumptions in this component-based approach influence the model's performance, and would affect model explainability as different geometric and design parameters may be better correlated to a specific loss assessment methodology [153,192]. The damage information was then translated into the repair cost of individual components and was aggregated to derive building repair cost at the building level for different shaking intensities. Lastly, the conditional distribution of losses over shaking intensities was integrated over site's hazard to derive expected annual loss (EAL)[193,194].

#### **4.3.2. Model development**

A training workflow comprising wrapper-based feature selection, hyperparameter tuning, and model evaluation was used to train several ML algorithms to estimate annual seismic loss in terms of repair costs based on building geometry and design parameters [195]. These "design-oriented" models aim to use only information available during the design process (early or detailed), providing opportunities for an ML-based performance design through inverse problem formulation [196]. A  $k$ -fold cross-validation approach was used to tune hyperparameters and evaluate models, where models are developed based on  $k-1$  sets and tested on the  $k$ th set for  $k$  iterations.

An ensemble learning-based model, random forest (RF), was selected to estimate EALs for different buildings. RF uses the result of a number of weak learners based on bagging (parallel learning). RF decorrelated constituent weak learners (i.e., decision stumps) by training each stump on bootstrapped sample of training data and separate feature set.

Figure 8 shows the performance of the RF model in predicting EAL for both the training and test sets. For the test set, the RF model achieved an  $R^2$  value of 0.90, 0.99, 0.99, and 0.99 for structural, nonstructural acceleration-sensitive, nonstructural drift-sensitive, and total losses. Overall, the model exhibits higher accuracy for nonstructural drift- and acceleration-sensitive losses than structural losses. Since nonstructural losses largely govern the total loss, the model's performance for total loss achieves similar accuracy as nonstructural loss predictions.



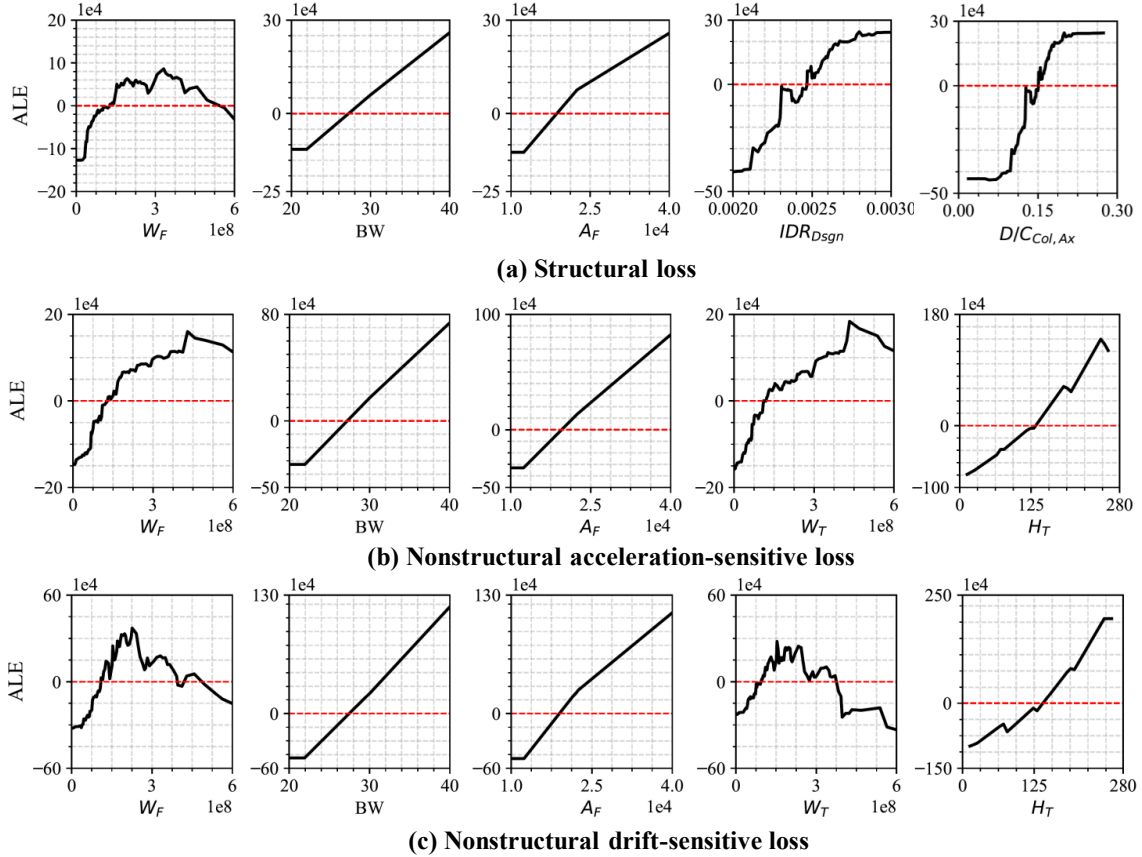
**Figure 8. Top:** RF performance on training and testing set to predict structural, nonstructural acceleration- sensitive, nonstructural drift-sensitive, demolition, and total expected annual loss. **Bottom:** Feature importance of each model

Figure 8 also includes feature importance plots highlighting that weight-related parameters, such as total ( $W_T$ ) and frame weights ( $W_F$ ), as well as floor area ( $A_F$ ), are influential across all assembly types. Furthermore, parameters critical for predicting nonstructural losses—including total and frame weight, bay width (BW),  $A_F$ , and building height ( $H_T$ )—also drive the total loss prediction. In contrast, structural losses are primarily influenced by two design-specific parameters: the demand-to-capacity ratio of column ( $D/C_{col,ax}$ ) and design drift ( $IDR_{Dsgn}$ ). This observation aligns with engineering principles, as nonstructural losses are governed by the quantity and characteristics of components, which are directly calculated based on building geometry in FEMA P-58 method for structures with similar systems and occupancy in a given location.

#### 4.3.3. Explainability techniques

ALE was selected to explain the RF models. Figure 9 shows a clear linear relationship between structural losses and bay width and floor area. In contrast, the relationships between structural losses and design drift and the column axial demand-to-capacity ratio are nonlinear and increasing. Structural losses are particularly sensitive to design drift values around 0.25% and demand-to-capacity ratios that are about 15%. Additionally, the relationship between frame weight and structural losses is nonlinear, with losses being lower than average at both extremes of the frame weight spectrum.

The most influential parameters are identical for nonstructural acceleration-sensitive losses and total losses. This observation is intuitive, as acceleration-sensitive nonstructural losses dominate total losses. Both floor area and bay width demonstrate a linear increase in relation to these loss types, while the relationships between weight-related parameters (frame and total weight) and losses are nonlinear. Specifically, the relationship is increasing for nonstructural acceleration-sensitive losses, but for total losses, it peaks at mid-range frame weights and then decreases, suggesting that frames with intermediate weights incur higher-than-average total losses. Overall, weight-related parameters, building height, and floor area significantly impact all loss types, with losses generally increasing as these parameters increase.



**Figure 9.** ALE results for (a) structural, (b) nonstructural acceleration-sensitive, and (c) total loss.

## 5. Challenges and Future Directions for XML in Structural Engineering

1. Current research efforts are directed at post-hoc explainability, with limited attention given to ante-hoc explainability (i.e., models with transparent inner workings). Future research should aim to develop and apply interpretable models, such as decision trees, to combine explainability with improved accuracy, thereby further enhancing their acceptance in structural engineering practice.
2. Most studies only use a single XML method, typically SHAP, for explaining ML models, with few comprehensive comparisons of different XML techniques for structure-level assessment. To advance the field, future research should prioritize systematic comparisons of various XML methods to better understand their strengths and limitations, and conduct benchmark studies to compare the performance of various XML techniques.
3. The current literature of XML for structural design and assessment against natural hazards emphasizes capacity, response/demand, and loss assessment, while resilience assessment remains largely unexplored. Future research should also apply and benchmark XML techniques across a broader spectrum of structural and geotechnical applications, particularly in areas like wind, fire, and soil-structure interaction, evaluations of recovery processes, and decision-making in multi-hazard scenarios.

4. Future research should also explore more computationally efficient XML algorithms, such as sampling-based approximations or surrogate modeling, and evaluate trade-offs between fidelity and computational feasibility.
5. XML techniques are often not intended to provide causal explanations. While non-causal explanations are still useful, understanding causal relationships would benefit many engineering problems, enabling more generalizable ML solutions [197–199]. Future efforts can focus on integrating causal inference methods with XML frameworks to communicate causal relationships. For example, investigation of the distinction between correlation-based explanations and causal reasoning; The reason that causal explanations are important to provide insights in engineering design and risk mitigation; and case study of applications that causal XML integration is helpful to understand the effects of design on fragility.
6. Most existing XML studies are based on the ML models trained on relatively small or synthetic datasets. However, the performance of XML techniques relies on the predictive performance of ML models. In this regard, the quality of training data and the robustness/generalization of ML models have been a concern for XML-based structure-level assessment. Compared to material-level and component-level data, structure-level experimental data is significantly less documented due to high costs, limiting its availability for ML-based assessments. While monitoring data is more abundant, challenges arise from the limited occurrence of extreme hazard events and restricted data accessibility, which can impact the reliability of ML models. To enhance the robustness and generalization of ML models for structural assessment, future efforts should focus on developing extensive, high-quality databases. Ideally, the databases incorporate experimental, field monitoring, and post-event reconnaissance data.

## 6. Conclusion

This paper reviews XML definitions, classifications, and techniques, and provides three structural-engineering-specific examples at different scales to illustrate how XML can connect the purely data-driven nature of ML to engineering-oriented insights. Despite the lack of a universal definition for XML, the literature often distinguishes between “interpretability” and “explainability”. In this context, interpretable models are designed to be inherently understandable, whereas explainability refers to post-hoc explanations of a complex model through different local and global techniques. Different taxonomies exist to classify XML techniques, including functionality-based, conceptual, result-based, and mixed taxonomies; however, none of these taxonomies have been tailored to the domain-specific needs of structural design and assessment. A targeted literature review of structural engineering literature demonstrates the dominance of post-hoc explainability through global model-agnostic methods, particularly SHAP, across various applications.

Applying XML techniques to three case studies highlighted their potential to infer engineering-oriented insights beyond establishing a predictive or classification model. The component-level case study demonstrated the feasibility of utilizing ante-hoc XML to provide more accurate and robust predictive models. By generating highly accurate equations based on existing datasets, regardless of the size, it is possible to define the relationship between key input parameters and performance. Applying post-hoc explainability to structure- and inventory-level problems demonstrated its capability to identify critical features, as well as the type of relationship they may have with the target variable, across black-box models such as random forests and neural networks. For example, feature importance-based explanations suggested

that seismic intensity measures (particularly AvgSa) were more important than structure-related parameters for peak column drift. Additionally, increasing AvgSa, PGD, PGV, number of spans, and column axial load ratio increases columns' peak drift responses, whereas column diameter and reinforcement ratio had a negligible impact. Lastly, applying ALE to the steel inventory problem revealed a linear relationship between structural losses and bay width and floor area, as well as a nonlinear increasing trend between the same loss and the column axial demand-to-capacity ratio and design drift. Lastly, weight-related design parameters, building height, and floor area affect all types of losses.

The paper identifies several future research directions. Despite the overall similarity between the outcomes of different post-hoc explainability techniques, there exist instances where various techniques rank a specific feature differently, suggesting the need for benchmarked comparison across diverse XML techniques to understand their limitations. While XML has been, to some extent, applied for seismic assessment, its application to other hazards or multi-hazard decision-making remains largely underexplored. Lastly, the computational cost and scalability are another critical consideration for the practical adoption of XML techniques in structural engineering, particularly for complex ML models such as large-scale infrastructure inventories.

### Data Availability Statement

Some or all data, models, or code that support the findings of this study are available from the corresponding author upon reasonable request.

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