

A DAE SOLVER FOR COORDINATION OF FORCE-CONTROLLED HYBRID TESTING

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ABSTRACT

Hybrid testing couples numerical simulation with physical experiments by enforcing compatibility and equilibrium at the interface between numerical and physical substructures. This paper presents a force-controlled hybrid-testing formulation for stiff physical substructures coupled to nonlinear numerical substructures. Displacement compatibility is imposed directly on the numerical equations of motion to form a differential-algebraic equation. The interface force is obtained using Baumgarte-stabilized Lagrange multipliers. Physical-substructure velocity and acceleration, which are not measured in pseudo-dynamic testing, are estimated from displacement feedback using cubic regression. A linear illustrative system is used to assess convergence, stability, numerical damping, and period distortion for explicit time-integration schemes. The formulation is then demonstrated experimentally by coupling a stiff axially loaded steel-bar physical substructure to a nonlinear two-degree-of-freedom numerical substructure with a Duffing-type spring. The hybrid test remains stable, reproduces the dominant simulated dynamics, and maintains displacement compatibility despite noisy camera-based displacement feedback. These results show that Baumgarte-stabilized constraint dynamics provide a practical strategy for force-controlled hybrid testing of stiff physical substructures.

INTRODUCTION

Background and motivation

Hybrid testing (HT) combines numerical simulation and physical testing by partitioning a system into a numerical substructure (NS) and a physical substructure (PS). Since its early developments (Hakuno et al. 1969; Takanashi et al. 1975), HT has been widely used in seismic testing (Pan et al. 2016; Saouma and Sivaselvan 2014) and extended to fire (Sauca et al. 2021), hydrodynamic (Neumann et al. 2023), aerodynamic (Su and Song 2019), and multi-actuator applications (Fermendois and Spencer 2017; Silva et al. 2020; Abbiati et al. 2015). When the PS response is rate independent, HT can be coordinated in a pseudo-dynamic regime, where the physical test is executed slower than the simulated structural response.

A coordination algorithm imposes force equilibrium and displacement compatibility between PS and NS. The commanded interface quantities and measured PS responses are realized through a transfer system that includes the actuators, sensors, controllers, and measurement systems. Coordination algorithms can be broadly classified as simulation-based or control-based. The former are derived from numerical schemes used to solve ODEs/DAEs (Kolay et al. 2015; Jia et al. 2011; Bursi et al. 2013), while the latter are formulated as solutions to control problems (Silva et al. 2020; Carrion and Spencer 2007; Liu et al. 2024a; Christenson et al. 2008; Ou et al. 2015; Zhou et al. 2017). Most HT implementations use displacement control, in which interface displacements are imposed on the PS and measured interface forces are returned to the NS.

Displacement control, however, becomes impractical when the PS stiffness is close to the loading-system stiffness, since small displacement disturbances propagate to large fluctuations in the restoring force. Several force-control strategies have therefore been proposed. (Dimig et al. 1999) introduced Effective Force Testing (EFT), in which a prescribed effective force reproduces the structural response to ground motion. (Zhao et al. 2005) improved EFT by adding velocity feedback compensation to increase force-tracking accuracy. (Forouzan and Nakata 2015) proposed a predictor–corrector algorithm for force-controlled HT to improve robustness and response accuracy. (Pratt and Krupp 2004) achieved force control by adding mechanical compliance to the actuator

51 loop, allowing the applied force to be inferred from Hooke's law, although this requires additional
52 physical hardware.

53 Other methods improve force tracking by modifying the actuator–structure interaction. Siva-
54 selvan et al. (Sivaselvan et al. 2008) proposed dynamic force control using added compliance
55 and displacement compensation through transfer-function representations. This approach was de-
56 veloped for stiff structures, but can become sensitive to changes in structural properties during
57 testing. (Carl and Sivaselvan 2011) addressed related stability issues using an impedance-based
58 feedforward scheme that modifies the apparent interaction between the actuator and the tested
59 structure. (Stefanaki and Sivaselvan 2018) proposed a simplified dynamic substructuring frame-
60 work that separates the substructuring controller from the PS dynamics and reduces the need for
61 explicit tracking-controller design. (Verma et al. 2019) introduced an impedance-matching strat-
62 egy in which the transfer system is modeled with a control input, a reaction-force input, and a
63 displacement output.

64 Although these methods have advanced force-controlled HT, many rely on linearized models
65 and transfer functions. Such assumptions are restrictive for nonlinear systems. As an example, this
66 limitation is increasingly relevant for geometric nonlinearities (Sauder et al. 2016; Belloli et al.
67 2020; Zhang et al. 2016; Ding et al. 2024; Herrmann et al. 2012; Shao et al. 2021; Guo et al. 2021;
68 Liu et al. 2024b; Witteveen and Koller 2023) and material nonlinearities (Chen et al. 2024; Bas
69 and Moustafa 2020a; Bas and Moustafa 2020b; Mosqueda and Ahmadizadeh 2011).

70 **Scope**

71 This paper proposes a DAE-based coordination algorithm for hybrid testing with force-controlled
72 physical-substructure degrees of freedom. DAE solvers provide an alternative by coupling the PS
73 and NS ODEs through algebraic interface constraints (Jia et al. 2011; Bursi et al. 2013). In the
74 proposed approach, displacement compatibility is imposed directly on the equations of motion of
75 the NS, and the corresponding interface force is obtained from Baumgarte-stabilized Lagrange
76 multipliers. The computed interface force is used as the actuator force command, enabling force-
77 controlled coupling of a stiff PS to a nonlinear NS. The formulation is assessed using a linear

78 stability benchmark and demonstrated experimentally on a stiff physical substructure coupled to a
79 nonlinear numerical model.

80 The paper is organized as follows. Section 1 introduces the background, motivation, and scope.
81 Section 2 presents the force-controlled hybrid-testing framework. Section 3 introduces a linear
82 illustrative hybrid system used to examine the numerical behavior of the proposed formulation.
83 Section 4 presents the convergence, stability, damping, and period-distortion analyses. Section 5
84 describes the nonlinear hybrid-test case study with a stiff physical substructure and the correspond-
85 ing experimental setup. Section 6 presents and discusses the hybrid-test results, and Section 7
86 concludes the work.

87 **FORCE-CONTROLLED HYBRID TESTING FRAMEWORK**

88 A coordination algorithm, which may take the form of either a time-integration or control
89 scheme, ensures that the NS and PS satisfy equilibrium and coupling conditions (for example,
90 displacement compatibility). In most work related to HT, the PS is tested in displacement-control
91 mode, as illustrated in Fig. 1a.

92 When coordinating an HT in displacement-control mode, the interface displacement between
93 the NS and PS is obtained at each timestep by integrating the state-space equations of the NS using
94 a suitable time-integration scheme. This displacement is sent as a command to an actuation system,
95 ensuring displacement compatibility between the two substructures. On the PS, the corresponding
96 interface force is measured using load cells and subsequently fed back to the NS to maintain
97 equilibrium in the next timestep.

98 Displacement control is, however, problematic when the PS has a stiffness of a similar order of
99 magnitude as that of the actuation system. In such configurations, the interface is typically subjected
100 to small displacements, making the compliance of the actuation system and the noise in feedback
101 signals non-negligible. Testing the PS in force-control mode addresses this issue. In this mode,
102 a force command is sent to the actuation system, and once equilibrium is achieved, the measured
103 displacement feedback is returned to the NS. The coordination algorithm for force-controlled HT
104 is illustrated in Fig. 1b.

105 Despite its advantages for testing a stiff PS, force-control introduces several difficulties for the
 106 NS. In this configuration, the NS receives a displacement and must return a corresponding force,
 107 which cannot be solved with forward dynamics. This inverse problem is often addressed using
 108 transfer functions, but such an approach is difficult for nonlinear systems.

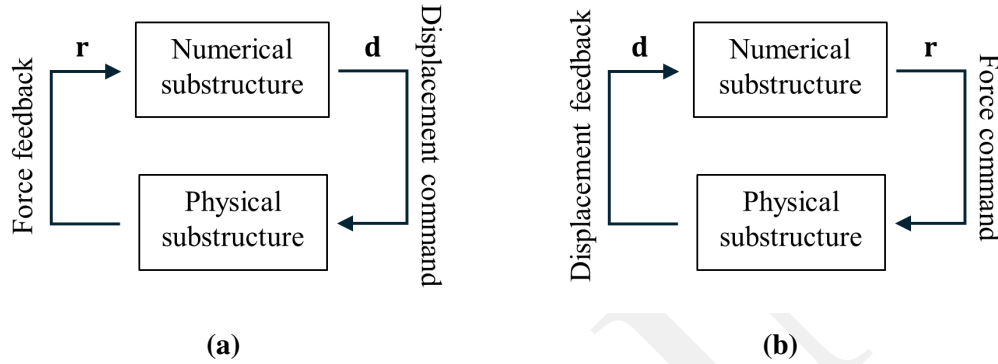


Fig. 1. Hybrid testing schemes for (a) displacement control mode and (b) force control mode.

109 In this setting, displacement compatibility is enforced as an algebraic constraint on the NS
 110 dynamics to form the differential algebraic equation (DAE),

$$\begin{cases} \dot{\mathbf{q}} = \mathbf{u} \\ \dot{\mathbf{u}} = -\mathbf{M}^{-1}(\mathbf{f} - \mathbf{h}^T \mathbf{r}) \\ \mathbf{h}\mathbf{q} = \mathbf{d} \end{cases} \quad (1)$$

112 where \mathbf{q} contains the NS degrees of freedom, \mathbf{u} contains their time derivatives, t is the simulation
 113 time, and (\cdot) denotes a time derivative. The mass matrix \mathbf{M} and force vector \mathbf{f} are obtained from
 114 the NS dynamics. The matrix \mathbf{h} maps the NS degrees of freedom to the PS displacement feedback
 115 \mathbf{d} , and \mathbf{r} is the interface force represented by the Lagrange multiplier.

116 Various numerical methods exist for solving DAEs. (Ascher and Petzold 1998) suggest, among
 117 others, half-implicit and projection-based Runge-Kutta methods. However, these approaches are
 118 effective only for prescribed constraints, as they exhibit instability within feedback loops. Addi-
 119 tionally, they often require iterations, which is problematic in a HT setting. Alternatively, as the

120 system in (1) is a Hessenberg index-2 DAE, it can be reduced to an ODE. Here the index refers
 121 to the number of differentiations required to eliminate algebraic constraints. Differentiating the
 122 constraint yields,

$$123 \quad \mathbf{g} = \mathbf{h}\mathbf{q} - \mathbf{d} = \mathbf{0} \quad (2)$$

$$125 \quad \dot{\mathbf{g}} = \mathbf{h}\mathbf{u} - \dot{\mathbf{d}} = \mathbf{0} \quad (3)$$

$$127 \quad \ddot{\mathbf{g}} = -\mathbf{h}\mathbf{M}^{-1}\mathbf{f} + \mathbf{h}\mathbf{M}^{-1}\mathbf{h}^T\mathbf{r} - \ddot{\mathbf{d}} = \mathbf{0} \quad (4)$$

128 From (4), the interface force \mathbf{r} can be determined and substituted into (1), reducing the system to
 129 an ODE. However, enforcing the constraint at the acceleration level alone leads to drift in position
 130 and velocity, compromising stability over time.

131 To mitigate this issue, Baumgarte stabilization (Baumgarte 1972) is applied by forming a
 132 weighted combination of (2), (3), and (4),

$$133 \quad \ddot{\mathbf{g}} + 2\alpha\dot{\mathbf{g}} + \beta^2\mathbf{g} = \mathbf{0} \quad (5)$$

134 where α and β are Baumgarte stabilization parameters that control the damping and stiffness of
 135 the constraint-error correction. Choosing $\alpha = \beta$ gives critical damping of the error dynamics.
 136 Larger values reduce constraint drift more aggressively, but can also increase sensitivity to noisy
 137 displacement feedback. The Baumgarte parameters are therefore treated as algorithmic tuning
 138 parameters rather than physical properties of the system, and their influence is examined explicitly
 139 in the stability analysis.

140 Solving for the interface force \mathbf{r} in (5) yields the expression,

$$141 \quad \mathbf{r} = -(\mathbf{h}\mathbf{M}^{-1}\mathbf{h}^T)^{-1}(-\mathbf{h}\mathbf{M}^{-1}\mathbf{f} - \ddot{\mathbf{d}} + 2\alpha(\mathbf{h}\mathbf{u} - \dot{\mathbf{d}}) + \beta^2(\mathbf{h}\mathbf{q} - \mathbf{d})) \quad (6)$$

142 which is used to transform the DAE of (1) into the ODE,

$$143 \quad \begin{cases} \dot{\mathbf{q}} = \mathbf{u} \\ \dot{\mathbf{u}} = -\mathbf{M}^{-1}(\mathbf{f} + \mathbf{h}^T(\mathbf{h}\mathbf{M}^{-1}\mathbf{h}^T)^{-1}(-\mathbf{h}\mathbf{M}^{-1}\mathbf{f} - \dot{\mathbf{d}} + 2\alpha(\mathbf{h}\mathbf{u} - \dot{\mathbf{d}}) + \beta^2(\mathbf{h}\mathbf{q} - \mathbf{d}))) \end{cases} \quad (7)$$

144 Introducing the state vector $\mathbf{x} = \begin{bmatrix} \mathbf{q} & \mathbf{u} \end{bmatrix}^T$, (7) can be reduced to,

$$145 \quad \dot{\mathbf{x}} = \mathbf{f}^{rhs}(\mathbf{x}, \mathbf{d}, \dot{\mathbf{d}}, \ddot{\mathbf{d}}) \quad (8)$$

146 However, in pseudo-dynamic hybrid tests, the velocity $\dot{\mathbf{d}}$ and acceleration $\ddot{\mathbf{d}}$ of the PS are not
147 directly measurable and must therefore be estimated. At each time step i , a cubic polynomial
148 approximation of the PS displacement is constructed.

149 Algorithm 1 summarizes the proposed force-controlled coordination scheme. Each Runge–
150 Kutta stage uses the available PS displacement history to estimate $\dot{\mathbf{d}}$ and $\ddot{\mathbf{d}}$, compute the Baumgarte-
151 stabilized interface force, send the force command, and update the NS state from the returned
152 displacement feedback.

Algorithm 1 Force-Controlled Hybrid Testing with Runge–Kutta Integration

- 1: Initialize $\mathbf{q}_1, \dots, \mathbf{q}_n, \mathbf{u}_1, \dots, \mathbf{u}_n$ and $\mathbf{d}_1, \dots, \mathbf{d}_n$
 - 2: **for** $i = n, n + 1, \dots, N_{\text{steps}} - 1$ **do** ▷ Time-stepping loop
 - 3: **for** $j = 1, \dots, s$ **do** ▷ Runge–Kutta stages loop
 - 4: Update stage $\mathbf{x}_{i,j} \leftarrow \mathbf{x}_i + \Delta t \sum_{k=1}^{j-1} a_{j,k} \dot{\mathbf{x}}_{i,k}$
 - 5: Estimate velocity $\dot{\mathbf{d}}_{i,j}(t_i)$ and acceleration $\ddot{\mathbf{d}}_{i,j}(t_i)$
 - 6: Compute interface force $\mathbf{r}_{i,j}$ from (6)
 - 7: Send force command $\mathbf{r}_{i,j}$
 - 8: Get displacement feedback $\mathbf{d}_{i,j}$
 - 9: Evaluate stage derivative $\dot{\mathbf{x}}_{i,j}$ from (8)
 - 10: **end for**
 - 11: Update state $\mathbf{x}_{i+1} \leftarrow \mathbf{x}_i + \Delta t \sum_{j=1}^s b_j \dot{\mathbf{x}}_{i,j}$
 - 12: Update time $t_{i+1} \leftarrow t_i + \Delta t$
 - 13: **end for**
-

153 Here, s denotes the number of Runge–Kutta stages, $a_{j,k}$ and b_j are the Runge–Kutta coefficients,
154 N_{steps} is the number of time steps, and n is the number of previous states retained for the derivative

155 estimation.

156 The PS is engaged at every Runge–Kutta stage, producing multiple force–displacement ex-
157 changes within each timestep. Since the force command depends on the measured displacement
158 feedback and the estimated PS derivatives, the stability and accuracy depend on the integration
159 scheme, Baumgarte parameters, and regression procedure. These effects are examined next using
160 a linear illustrative hybrid system.

161 LINEAR ILLUSTRATIVE HYBRID TEST

162 To illustrate the proposed framework, a simple linear hybrid system is considered. The system
163 consists of a single-degree-of-freedom NS coupled to a PS, as shown in Fig. 2. The NS comprises
164 a rigid body of mass $m = 0.5 \text{ kg}$ attached to a linear spring with stiffness $k_{NS} = 0.39 \text{ N/mm}$, where
165 the motion is described by the translational degree of freedom q . The PS is connected to the mass
166 and is realized as a spring.

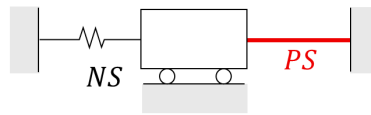


Fig. 2. Hybrid model of illustrative example, where the physical substructure (PS) is highlighted in red and the numerical substructure is in black.

167 The hybrid model is decomposed into a free-body diagram, revealing the interface force. The
168 resulting NS and PS are shown in Fig. 3a and -b, respectively.

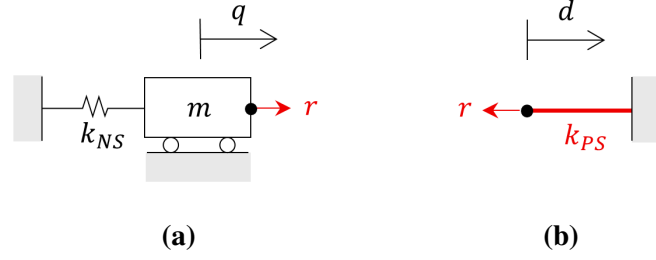


Fig. 3. Free-body diagrams of the decomposed linear hybrid model showing a) the numerical substructure and b) the physical substructure. The interface force r couples the two substructures, and d is the PS displacement.

169 The dynamics of the NS are governed by,

$$170 \quad m\ddot{q} + k_{NS}q - r = 0 \quad (9)$$

171 where r is the interface force between NS and PS, represented by a Lagrange multiplier to ensure
 172 displacement compatibility $q = d$ between the two substructures.

173 Following the form of (1), the coupled system is written as,

$$174 \quad \begin{cases} \dot{q} = u \\ \dot{u} = -\frac{1}{m}(k_{NS}q - r) \\ q = d \end{cases} \quad (10)$$

175 Applying the Baumgarte-stabilized constraint solution from (6) gives the interface force,

$$176 \quad r = k_{NS}q + m(\ddot{d} + 2\alpha(\dot{d} - u) + \beta^2(d - q)) \quad (11)$$

177 Substitution of (11) into (10) eliminates the algebraic constraint and gives the ODE,

$$178 \quad \begin{cases} \dot{q} = u \\ \dot{u} = \ddot{d} + 2\alpha(\dot{d} - u) + \beta^2(d - q) \end{cases} \quad (12)$$

179 For this single-DOF example, the mechanical dynamics are absorbed into the interface force. The
 180 remaining ODE describes a stabilized constraint motion, with PS-acceleration feedforward and
 181 PD-type Baumgarte correction of the compatibility error.

182 The linear illustrative example is evaluated numerically using a virtual PS. The PS is modeled
 183 as a linear spring, such that,

$$184 \quad d = \frac{1}{k_{PS}} r \quad (13)$$

185 where $k_{PS} = 0.39$ N/mm is the assumed PS stiffness.

186 The proposed framework is not restricted to a specific scheme for estimating the PS velocity
 187 and acceleration. In the present implementation, these quantities are obtained from a local cubic
 188 reconstruction of the displacement history,

$$189 \quad \dot{d}_i = \left. \frac{d}{dt} f(t) \right|_{t=t_i}, \quad \ddot{d}_i = \left. \frac{d^2}{dt^2} f(t) \right|_{t=t_i} \quad (14)$$

190 where i denotes the current timestep and $f(t)$ is the reconstructed PS displacement trajectory, given
 191 by,

$$192 \quad f(t) = a_3 t^3 + a_2 t^2 + a_1 t + a_0 \quad (15)$$

193 The coefficients a_3 , a_2 , a_1 and a_0 are determined by a least-squares fit to the known displacement
 194 and velocity history, augmented by the current displacement,

$$195 \quad \mathbf{a} = \arg \min_{\mathbf{a}} \left\| \begin{bmatrix} f(t_i) - d_i \\ f(t_{i-1}) - q_{i-1} \\ f(t_{i-2}) - q_{i-2} \\ f(t_{i-3}) - q_{i-3} \\ \frac{d}{dt} f(t_{i-1}) - u_{i-1} \\ \frac{d}{dt} f(t_{i-2}) - u_{i-2} \\ \frac{d}{dt} f(t_{i-3}) - u_{i-3} \end{bmatrix} \right\| \quad (16)$$

196 where $\mathbf{a} = [a_3 \ a_2 \ a_1 \ a_0]^T$. The resulting history-dependent discrete-time formulation is ex-
 197 pressed through the augmented state vector,

$$198 \quad \mathbf{x}_i = \left[\mathbf{q}_{i-1} \ \dots \ \mathbf{q}_{i-n} \ \mathbf{u}_{i-1} \ \dots \ \mathbf{u}_{i-n} \ \mathbf{d}_i \right]^T \quad (17)$$

199 Here, $n = 3$ is used. The augmented state captures the finite memory introduced by the regression-
 200 based estimation of the PS velocity and acceleration, enabling a discrete stability analysis of the
 201 coordination algorithm.

202 STABILITY ANALYSIS

203 The force-controlled coordination algorithm creates a feedback loop between the NS and the
 204 measured displacement of the PS. The stability of the hybrid test therefore depends on the time-
 205 integration scheme, the Baumgarte stabilization parameters, and the estimation of PS velocity and
 206 acceleration. These effects are examined using the linear illustrative system as a benchmark.

207 Fig. 4 verifies the convergence of the coordination algorithm for different time-integration
 208 schemes. The global truncation error is evaluated after 1 ms by comparison with a reference
 209 solution computed using a sufficiently small timestep.

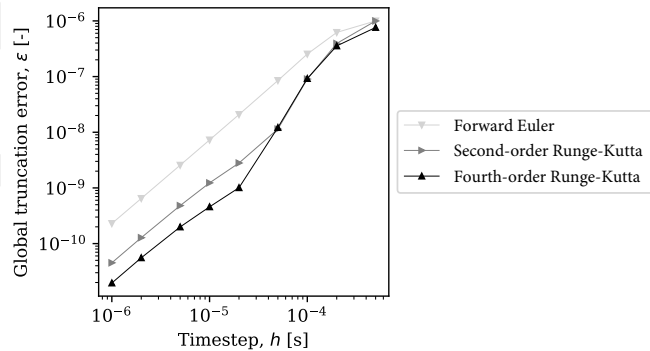


Fig. 4. Convergence analysis of global truncation error for 1 ms simulation.

210 The convergence results in Fig. 4 show decreasing global error for all integration schemes
 211 as the timestep is reduced. For standalone integration, the expected global errors are $O(h)$ for
 212 forward Euler, $O(h^2)$ for RK2, and $O(h^4)$ for RK4. These nominal orders are not recovered, as the

213 error curves have approximately the same slope. Thus, convergence is governed by the complete
 214 coordination algorithm rather than by the time integrator alone. The Baumgarte feedback terms
 215 and cubic regression of PS velocity and acceleration introduce additional approximation errors in
 216 the force command. These errors limit the practical benefit of higher-order Runge–Kutta schemes
 217 for the force-controlled hybrid test.

218 To determine the stable timestep range, the coordination algorithm is written as a discrete
 219 state-transition map. Stability is assessed from the spectral radius of the state-transition matrix \mathbf{A} ,

$$220 \quad \rho(\mathbf{A}) = \|\mathbf{A}\|_\infty = \max_j |\lambda_j|, \quad j = 1, \dots, n_\lambda \quad (18)$$

221 where $\mathbf{\Lambda} = \text{diag}(\lambda_1, \lambda_2, \dots, \lambda_{n_\lambda})$ contains the eigenvalues of \mathbf{A} . The eigenvalues and eigenmodes
 222 are obtained from,

$$223 \quad \mathbf{A}\boldsymbol{\varphi}_j = \lambda_j\boldsymbol{\varphi}_j, \quad (19)$$

224 where $\boldsymbol{\varphi}_j$ is the eigenmode associated with λ_j . The matrix \mathbf{A} is the linearized state-transition
 225 matrix,

$$226 \quad \mathbf{A} = \frac{\partial \mathbf{x}_{i+1}}{\partial \mathbf{x}_i}. \quad (20)$$

227 The spectral radius gives the stability limit, but not the numerical damping or phase error within
 228 the stable range. These quantities are extracted by expressing the dominant complex eigenvalues
 229 as,

$$230 \quad \lambda = \exp(\omega\Delta t(-\xi \pm i)) \quad (21)$$

231 where $i = \sqrt{-1}$, and ω and ξ are the algorithmic frequency and damping ratio, respectively. The
 232 damping ratio ξ quantifies numerical amplitude decay, while the algorithmic frequency ω is used
 233 to compute the period distortion,

$$234 \quad \Delta T = \frac{2\pi}{\omega} - T_0, \quad (22)$$

235 where T_0 is the period of the first eigenfrequency.

236 The spectral radius, numerical damping, and period distortion are shown in Fig. 5, 6 and 7,

237 respectively, evaluated for different time-integration schemes as a function of the nondimensional
 238 timestep,

$$239 \quad \Omega = \omega_0 \Delta t, \quad (23)$$

240 where ω_0 is the lowest eigenfrequency of the coupled system and Δt is the integration timestep.

241 In contrast to the convergence analysis, the stability results show a clear difference between
 242 the integration schemes. In Fig. 5, instability occurs when $\rho > 1$. The Forward Euler scheme
 243 exceeds this limit even at the smallest timestep considered, whereas RK2 and RK4 remain stable
 244 for approximately $\Omega < 0.25$. The same trend is observed in Fig. 6, confirming the poor stability of
 245 Forward Euler and the larger stable timestep range of the Runge–Kutta schemes.

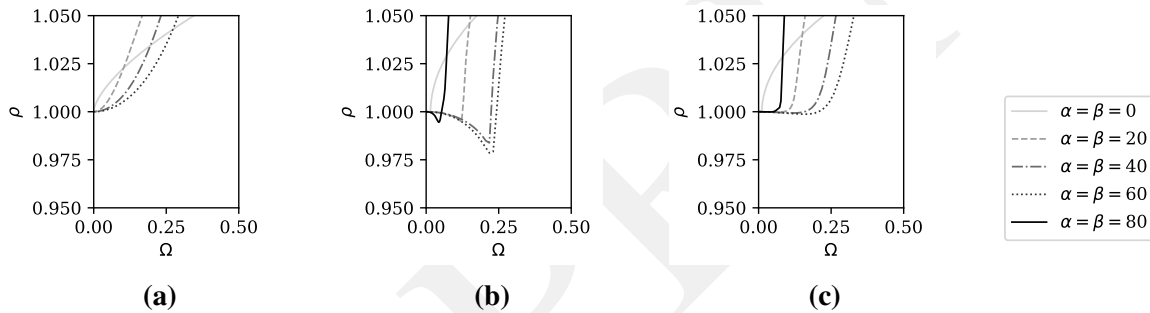


Fig. 5. Spectral radius, with instability occurring when $\rho > 1$ for a) forward Euler, b) second-order Runge–Kutta and c) fourth-order Runge–Kutta.

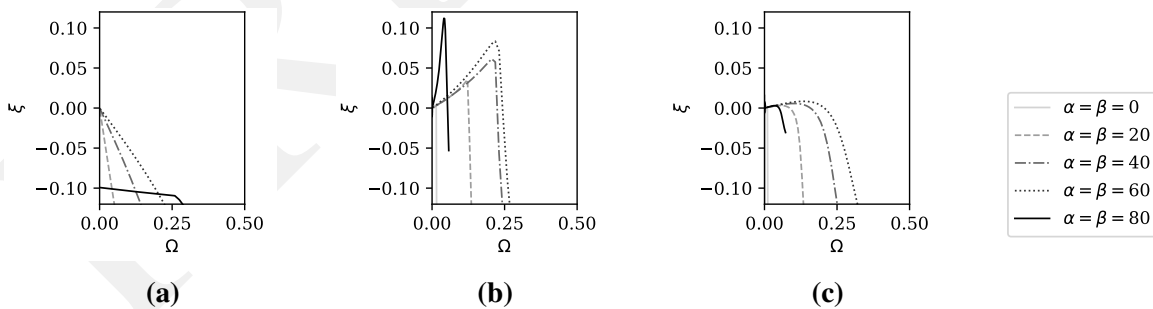


Fig. 6. Algorithmic damping ratio for a) forward Euler, b) second-order Runge–Kutta and c) fourth-order Runge–Kutta.

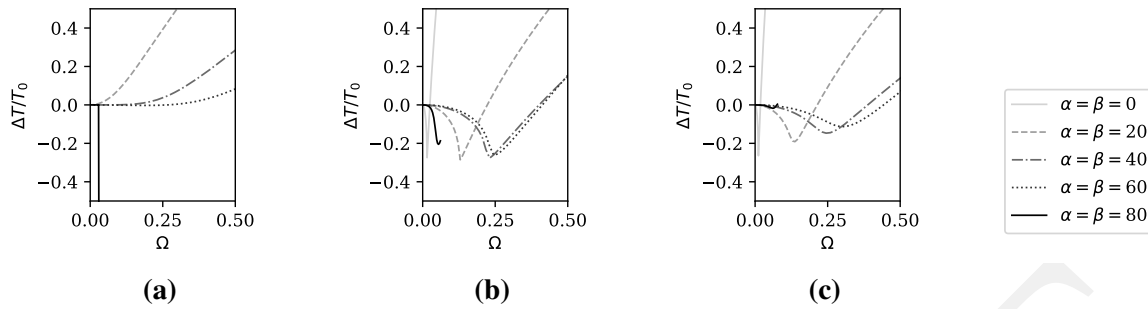


Fig. 7. Algorithmic period distortion for a) forward Euler, b) second-order Runge–Kutta and c) fourth-order Runge–Kutta.

246 Although RK2 and RK4 have similar stability limits, RK4 introduces less algorithmic dissipation,
 247 as shown in Fig. 6, and less algorithmic period distortion, as shown in Fig. 7.

248 The Baumgarte stabilization parameters influence the stability range, algorithmic dissipation,
 249 and period distortion. The results suggest an optimal parameter range, where the numerical response
 250 is least modified. Since velocity and acceleration are estimated from displacement feedback using
 251 cubic regression, the feedback loop contains memory. The observed behaviour therefore reflects
 252 the complete coordination algorithm, not the time integrator alone.

253 CASE STUDY: HYBRID TEST WITH A NONLINEAR NUMERICAL 254 SUBSTRUCTURE AND STIFF PHYSICAL SUBSTRUCTURE

255 Hybrid model

256 A hybrid test is conducted for a system in which a stiff PS is coupled to a nonlinear multi-DOF
 257 NS. The hybrid model is shown in Fig. 8. The model is decomposed into the NS and PS shown in
 258 Figs. 9a and 9b, respectively. The substructures are coupled through the interface force r and the
 259 compatibility condition imposed on the PS elongation d . The NS is a two-DOF oscillator in which
 260 each DOF describes the horizontal displacement of a rigid body with mass m . A Duffing-type
 261 spring with linear stiffness k and cubic stiffness coefficient μ is connected to the first mass, while
 262 an external load f_{ext} is applied to the second mass. The PS connects the two masses and is realized
 263 by a steel bar loaded in axial extension.

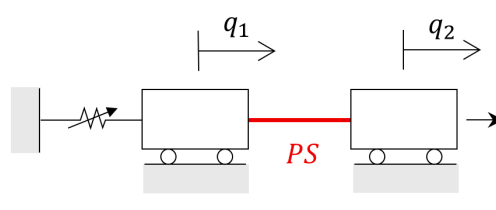


Fig. 8. Case-study hybrid model consisting of a nonlinear two-DOF NS coupled to a stiff steel-bar PS loaded in axial extension.

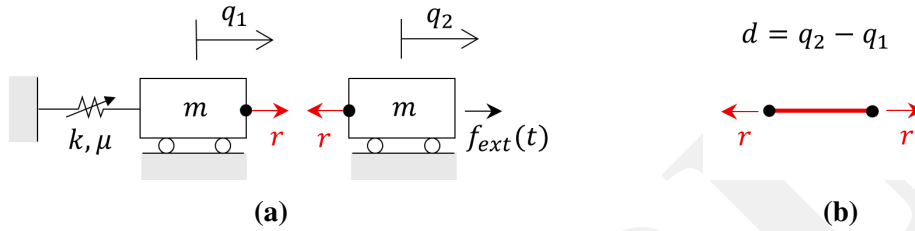


Fig. 9. Free-body diagrams of the decomposed hybrid model showing a) the numerical substructure and b) the physical substructure. The interface force r couples the two substructures, and the PS elongation d defines the displacement compatibility condition.

The PS elongation is imposed as the relative displacement between the two NS masses,

$$q_2 - q_1 = d \quad (24)$$

Using the DAE formulation in Eq. (1), the NS dynamics and interface constraint are specified by the mass matrix, force vector, and constraint mapping,

$$\mathbf{M} = \begin{bmatrix} m & 0 \\ 0 & m \end{bmatrix}, \quad \mathbf{f} = \begin{bmatrix} kq_1 + \mu q_1^3 \\ -f_{\text{ext}}(t) \end{bmatrix}, \quad \mathbf{h} = \begin{bmatrix} -1 & 1 \end{bmatrix} \quad (25)$$

where, for the case study, $m = 1000$ kg, $k = 2.33$ kN/mm, and $\mu = 36.60$ kN/mm³. The external force $f_{\text{ext}}(t)$ is applied to the second mass as a smooth pulse with a peak amplitude of 200 N at $t = 0.1$ s. These model properties define the numerical part of the hybrid test, while the stiff PS is realized experimentally as described next.

273 **Experimental setup**

274 The experimental setup is a two-degree-of-freedom hybrid test bench at Aarhus University as
275 shown in Fig. 10. The PS is a steel bar mounted in a support frame and connected to two orthogonal
276 electromechanical actuators. Only the horizontal actuator is used here, providing a simple test case
277 for force-controlled hybrid testing of a stiff PS.

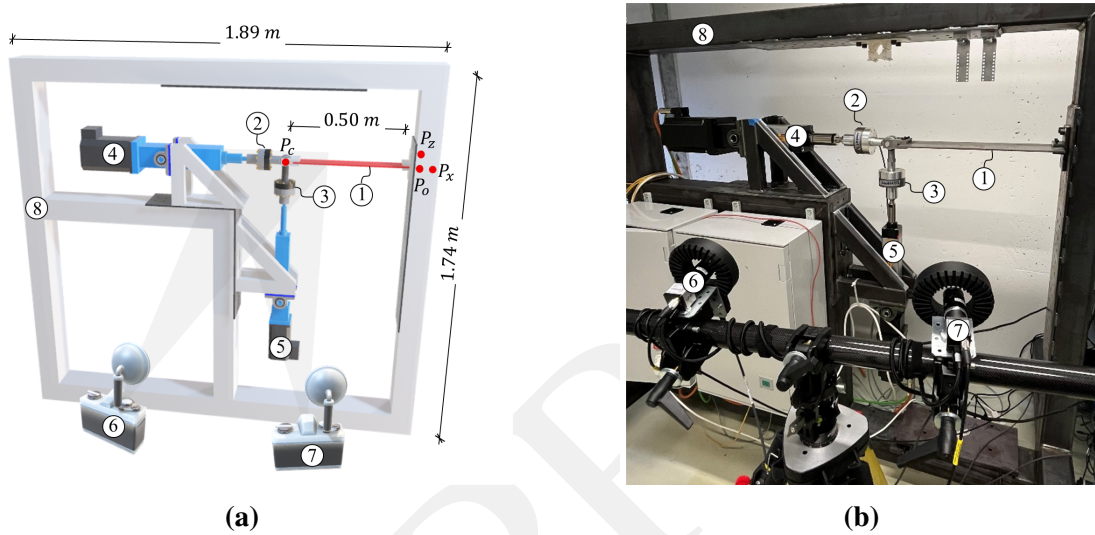


Fig. 10. Two-degree-of-freedom hybrid test bench at Aarhus University with 1) axially loaded steel bar serving as the physical substructure, 2) and 3) force transducers, 4) and 5) electromechanical actuators, 6) and 7) cameras for 3D measurement, and 8) support steel structure.

278 The test bench uses two orthogonally mounted electromechanical cylinders driven by servo-
279 motors. The horizontal actuator has a 5 mm screw lead and a nominal stroke of 100 mm; the
280 orthogonal actuator has the same screw lead and a nominal stroke of 200 mm. The steel bar has
281 length $l = 500$ mm and approximate axial stiffness $k_{PS} = 2.9$ kN/mm.

282 The setup is controlled by an INDEL SAM4 real-time controller. Actuator I/O is updated
283 through GinLink at 4 kHz, giving a control interval of 0.25 ms. Force control is implemented in
284 INOS as a proportional load-cell feedback loop, where the actuator velocity command is adjusted
285 to track the commanded interface force.

286 The hybrid-test timestep is $\Delta t = 0.5$ ms in simulation time. Because the test is pseudo-dynamic,
287 each step is executed more slowly in wall-clock time. Camera-based displacement feedback is

288 acquired once per step at an average wall-clock rate of approximately 0.3 Hz.

289 PS displacement is measured using a calibrated Linearis3D Video photogrammetry system.
 290 The marker positions P_0 , P_x , P_y , and P_c are reconstructed from synchronized camera images by
 291 triangulation (Linearis3D GmbH & Co. KG 2022). Markers P_0 , P_x , and P_y define the local
 292 reference frame, while the PS elongation d is obtained from the change in distance between P_0 and
 293 P_c .

294 The force-controlled hybrid-test implementation is summarized in Fig. 11. The NS computes
 295 the interface force command r from the measured PS elongation d . Although the electromechanical
 296 actuator is internally displacement controlled, the commanded interface force is tracked by the outer
 297 load-cell feedback loop.

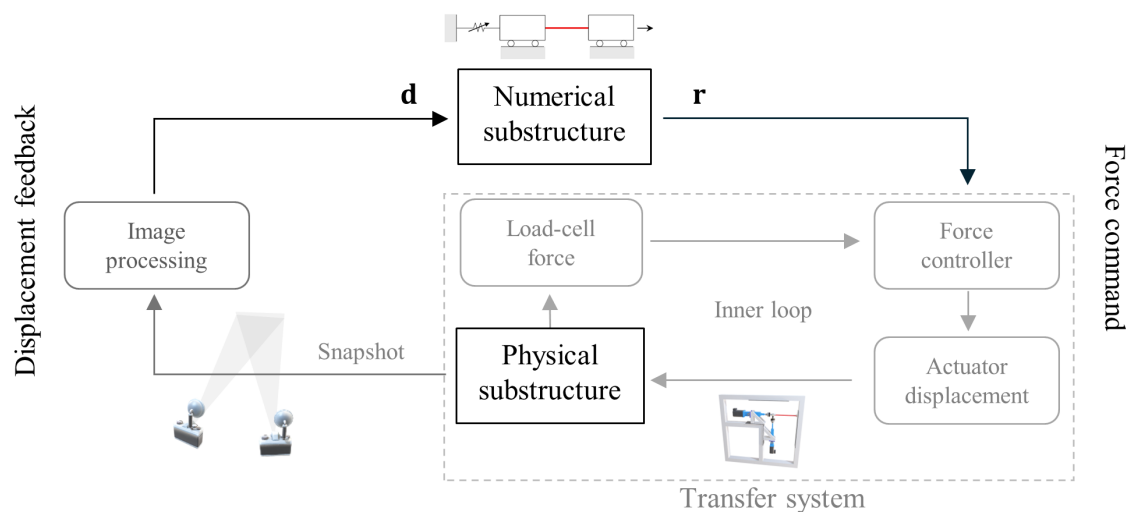


Fig. 11. Schematic overview of the force-controlled hybrid-test implementation. The NS computes the interface force command r , which is tracked by a load-cell feedback loop. The PS elongation d is obtained from camera snapshots and returned to the NS as displacement feedback.

298 The hybrid test used Baumgarte parameters $\alpha = \beta = 60 \text{ s}^{-1}$. PS derivatives were estimated
 299 by cubic regression using the three most recent simulated displacement–velocity states and the
 300 current measured displacement. The resulting coupled response, interface compatibility, and
 301 force–displacement behavior are evaluated next.

302

RESULTS AND DISCUSSION

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The hybrid-test response is evaluated through the NS motion, interface compatibility, and force–displacement behavior. Fig. 12 compares the NS coordinates q_1 and q_2 with the reference simulation, while Fig. 13 shows the measured PS elongation d against the relative NS displacement. The stiff-PS response is assessed from the force–displacement relation in Fig. 14, and the nonlinear NS behavior from the Duffing restoring-force relation in Fig. 15.

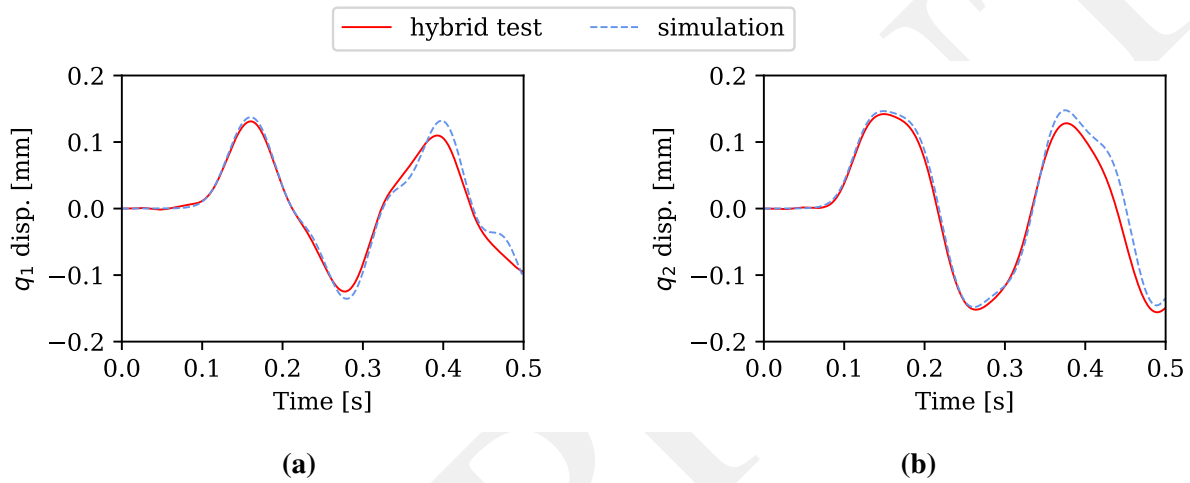


Fig. 12. Time response of the NS coordinates comparing the hybrid-test response with the reference simulation for a) displacement q_1 and b) displacement q_2 .

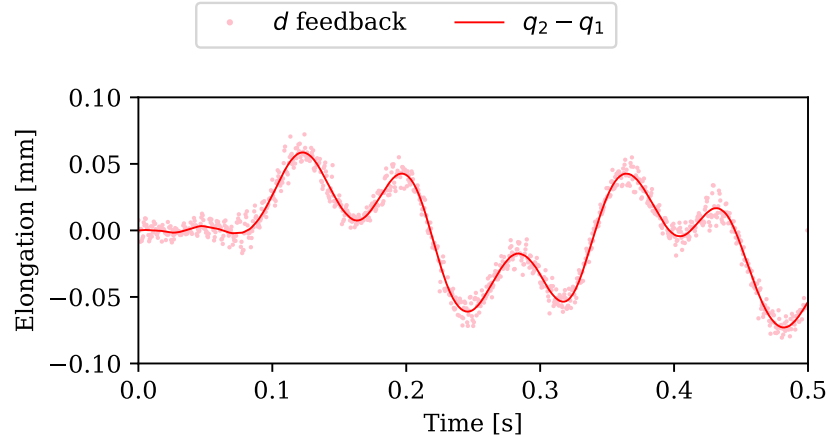


Fig. 13. Measured PS elongation during the hybrid test compared with the relative NS displacement $q_2 - q_1$.

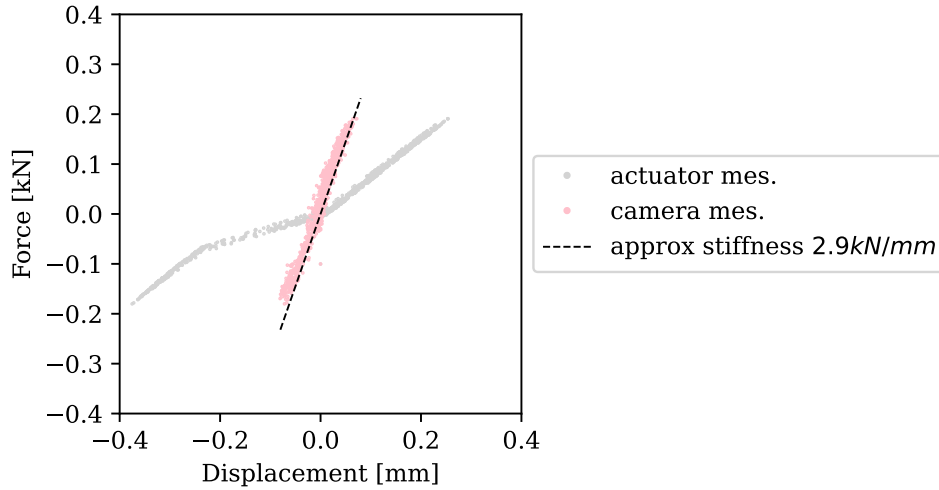


Fig. 14. Measured force–displacement response of the stiff PS comparing actuator-based displacement and camera-based specimen elongation.

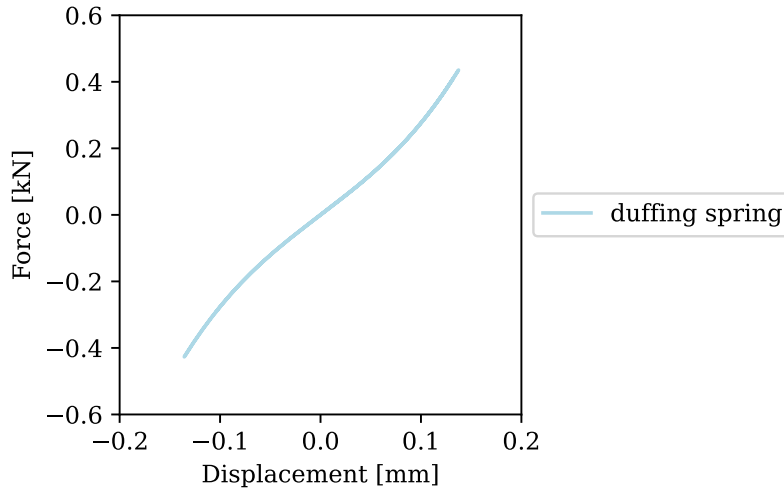


Fig. 15. Nonlinear force-displacement relation of the Duffing spring in the NS.

308 The behavior of the hybrid test is quantified using performance indices adapted from the hybrid
 309 testing criteria proposed in (Silva et al. 2020),

310

$$J_1 = \sqrt{\frac{\sum_{i=1}^N [\mathbf{h}\mathbf{q}(t_i) - d(t_i)]^2}{\sum_{i=1}^N [d(t_i)]^2}} = 17.18 \% \quad (26)$$

$$J_2 = \sqrt{\frac{\sum_{i=1}^N [q_1(t_i) - q_1^{\text{ref}}(t_i)]^2}{\sum_{i=1}^N [q_1^{\text{ref}}(t_i)]^2}} = 15.34 \% \quad (27)$$

$$J_3 = \sqrt{\frac{\sum_{i=1}^N [q_2(t_i) - q_2^{\text{ref}}(t_i)]^2}{\sum_{i=1}^N [q_2^{\text{ref}}(t_i)]^2}} = 17.68 \% \quad (28)$$

The hybrid test remains stable and reproduces the dominant dynamics of the reference simulation. The NS coordinates q_1 and q_2 in Fig. 12 show bounded oscillatory responses without visible amplitude growth. The hybrid-test trajectories reproduce the main phase and amplitude of the reference response, with normalized errors of $J_2 = 15.34\%$ for q_1 and $J_3 = 17.68\%$ for q_2 . These errors are considered acceptable for this proof-of-concept test because they remain bounded and do not introduce drift or instability. The mismatch is mainly attributed to displacement-feedback noise, derivative-estimation error, transfer-system compliance, and imperfect force tracking. This indicates that the force-controlled coupling preserves the main coupled response without destabilizing the hybrid system.

The Baumgarte-stabilized constraint introduces small feedback oscillations, but these do not destabilize the response. In Fig. 13, the measured PS elongation d follows the relative NS displacement $q_2 - q_1$, while showing small oscillations around the hybrid-test response. The normalized compatibility error is $J_1 = 17.18\%$, but no drift is observed during the test, and the oscillations do not propagate into growing amplitudes in q_1 or q_2 . This suggests that the Baumgarte terms act as a soft constraint correction. The compatibility error is regulated gradually, making the force command less sensitive to noise in the displacement feedback.

The actuator displacement does not represent the true elongation of the stiff PS. Fig. 14 shows a clear difference between the actuator-based and camera-based force–displacement curves. The camera measurement gives the local specimen elongation and follows an approximately linear stiffness of about 2.9 kN/mm, whereas the actuator displacement includes additional deformation from the transfer system. This shows that the transfer-system compliance is not negligible compared with the deformation of the stiff PS. Independent displacement feedback is therefore needed, and

337 the result supports the use of force control for stiff physical substructures.

338 The method couples the stiff experimental PS to a nonlinear NS. Fig. 15 shows the nonlinear
339 Duffing restoring-force relation, while Figs. 12 and 13 show that the coupled response remains
340 stable. This confirms that the formulation can handle nonlinear numerical-substructure dynamics.

341 The remaining mismatch reflects the sensitivity of the coordination scheme to timestep selec-
342 tion, Baumgarte stabilization, derivative estimation, and measurement noise. The timestep must
343 remain within the stable range identified in the linear analysis, while the Baumgarte parameters bal-
344 ance constraint enforcement against noise sensitivity. In addition, the cubic-regression derivative
345 estimates introduce approximation errors and may amplify noisy displacement feedback. These
346 coupled effects must therefore be tuned jointly.

347 CONCLUSION

348 This paper presented a DAE-based coordination method for force-controlled hybrid testing
349 of stiff physical substructures coupled to nonlinear numerical substructures. Displacement com-
350 patibility was imposed as an algebraic constraint, and the interface force was obtained using
351 Baumgarte-stabilized Lagrange multipliers.

352 Since physical-substructure velocity and acceleration are not measured directly in pseudo-
353 dynamic testing, they were estimated from displacement feedback. In the present implementation,
354 cubic regression introduced finite memory into the feedback loop. Consequently, the nominal con-
355 vergence orders of the standalone time integrators were not recovered, and stability and convergence
356 depended on the complete coordination algorithm.

357 The linear benchmark showed that the stable timestep range depends on the integration scheme,
358 Baumgarte parameters, and derivative estimation. For the selected parameters, RK2 and RK4
359 remained stable for approximately $\Omega < 0.25$, while Forward Euler was unstable over the investigated
360 range. RK4 introduced less numerical damping and period distortion than RK2.

361 The experimental test demonstrated stable force-controlled coupling between a stiff steel-bar
362 physical substructure and a nonlinear two-DOF numerical substructure. The response remained
363 bounded and maintained displacement compatibility without drift. Camera-based elongation mea-

364 surement was required because actuator displacement included transfer-system compliance.

365 Future work should address the joint effect of derivative estimation and Baumgarte stabilization
366 in pseudo-dynamic force-controlled hybrid tests.

367 CREDIT AUTHORSHIP CONTRIBUTION STATEMENT

368 **F. Nordtorp:** Writing – review & editing, Writing – original draft, Visualization, Investigation,
369 Formal analysis, Validation, Software, Methodology, Conceptualization. **G. Abbiati:** Writing –
370 review & editing, Project administration, Supervision, Methodology, Conceptualization.

371 DATA AVAILABILITY STATEMENT

372 Some or all data, models, or code that support the findings of this study are available from the
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